

**RESIDENTIAL / COMMERCIAL  
DEVELOPMENT  
460 KEIL DRIVE SOUTH  
CHATHAM, ON  
TRAFFIC IMPACT STUDY**



**RC SPENCER ASSOCIATES INC.**  
Consulting Engineers

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# RESIDENTIAL / COMMERCIAL DEVELOPMENT

## 460 KEIL DRIVE SOUTH, CHATHAM, ON

### TRAFFIC IMPACT STUDY (FEBRUARY 2021)

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## INTRODUCTION AND BACKGROUND

A residential / commercial development has been proposed for lands situated at 460 Keil Drive South in Chatham, Ontario. As may be noted on Figure 1, the site is located on the southwest corner of Keil Drive South at Park Avenue West. Keil Drive South is a major collector roadway that runs northwesterly from its origin south of Park Avenue West; it becomes a section of County Road 2 from Richmond Street to Grand Avenue West and then continues as a major collector until its terminus, about one kilometre north of Grand Avenue West. Richmond Street is a northeast / southwest arterial roadway that is also defined as County Road 2 west of Keil Drive South. Park Avenue West is a major northeast / southwest collector roadway that runs through Chatham. For the sake of simplicity, in this study, Richmond Street was assumed to run east/west, Keil Drive South was assumed to run north/south, and Park Avenue West was assumed to run east/west.

As illustrated on Figure 2, the study area includes the proposed eastbound stop-controlled site access at Keil Drive South, the northbound stop-controlled site access at Park Avenue West, and the signalized intersections of Richmond Street and Park Avenue West at Keil Drive South.

The proposed site plan, provided on Figure 3, is comprised of a total of 117 residential units in a single six-storey apartment building; the apartment building will also include approximately 5005 sq. ft. of ground floor commercial space. A total of 208 parking spaces are proposed to accommodate the entire development. Vehicular access is proposed via the site accesses on Park Avenue West and Keil Drive South, and a bus stop located on the west side of the Keil Drive South, immediately north of the proposed site access, will accommodate potential transit system users.

The purpose of this study is to examine, quantify, and qualify the potential traffic impact of the proposed development on area traffic operations; in particular, the study will investigate the potential effects of the development at the existing signalized intersections of Richmond Street and Park Avenue West with Keil Drive South.

## TRAFFIC DATA COLLECTION

Turning movement counts for the signalized intersections of Richmond Street at Keil Drive South and Park Avenue West at Keil Drive South were obtained by RC Spencer Associates Inc. on 15 and 16 December 2020, respectively. The collected turning movement counts are provided in Appendix A.

## METHODOLOGY

The aforementioned turning movement counts provided the basis for industry-standard traffic operations analysis; the software package utilized for the analysis (Synchro 10) calculates various parameters of intersection performance, such as level of service (LOS), intersection capacity utilization (ICU), control delay, and queue lengths on individual approaches.

Signalized level of service results are reported based on the following industry standard:

Level of Service	Average Control Delay (sec/veh)	General Description (Signalized Intersections)
A	≤10	Free Flow
B	>10 - 20	Stable Flow (slight delays)
C	>20 - 35	Stable flow (acceptable delays)
D	>35 - 55	Approaching unstable flow (tolerable delay, occasionally wait through more than one signal cycle before proceeding)
E	>55 - 80	Unstable flow (intolerable delay)
F	>80	Forced flow (jammed)

Unsignalized level of service results are reported based on the following industry standard:

Level of Service	Average Control Delay (sec/veh)
A	0 - 10
B	>10 - 15
C	>15 - 25
D	>25 - 35
E	>35 - 50
F	>50

## TRIP GENERATION AND DISTRIBUTION

Trip generation for the proposed development was estimated from the ITE Trip Generation Manual (10<sup>th</sup> Edition). The dataset's average rate was used instead of the fitted curve because the value of the independent variables is in the lower range of the dataset, and the fitted curve equation does not pass through the origin. It was determined that ITE Land Use Codes 221 (Mid-Rise Multifamily Housing) and 820 (Shopping Center) were the most appropriate land use codes for the proposed development. All trips to and from the proposed development site were assumed to be vehicle trips; the nearby bus stop may result in some modal split, but no reductions were considered in the calculations.

The following information was extracted from the ITE Trip Generation Manual:

- Land Use Code 221 (Mid-Rise Multifamily Housing) provides average generation rates of 0.36 trips per unit in the AM peak hour, with 26% entering and 74% exiting, and 0.44 trips per unit in the PM peak hour, with 61% entering and 39% exiting; and
- Land Use Code 820 (Shopping Center) provides average generation rates of 0.94 trips per 1000 sq. ft. GLA in the AM peak hour, with 62% entering and 38% exiting, and 3.81 trips per 1000 sq. ft. GLA in the PM peak hour, with 48% entering and 52% exiting.

The trip generation calculations are contained in Appendix B. When combined, the total trips generated by the proposed development are estimated to be 14 entering and 33 exiting during the AM peak hour and 40 entering and 31 exiting during the PM peak hour. Site generated traffic was distributed to and from the proposed site accesses at Keil Drive South and Park Avenue West. The distribution of intersection turning movements was based on the directional flow of existing traffic volumes on Keil Drive South and Park Avenue West, as taken from the collected turning movement counts. The resulting site generated turning movements are illustrated on Figure 4.

## CAPACITY AND LEVEL OF SERVICE ANALYSIS

Detailed Synchro 10 analysis was carried with respect to the following traffic scenarios:

- Existing Traffic;
- Existing + Site Generated Traffic;
- Total Traffic 2026 (Background Traffic 2026 + Site Generated Traffic); and
- Total Traffic 2031 (Background Traffic 2031 + Site Generated Traffic).

To be conservative, the analysis was carried out assuming full build-out conditions for existing, 2026 and 2031 horizon years. Background traffic was increased by 1% per year for the 2026 and 2031 horizon forecasts, which is conservative for the region. Figures 5 to 8 summarize total traffic estimates that result from adding the site generated traffic to the existing, 2026, and 2031 horizon year forecasts for background traffic in the study area. The effect of adding site generated traffic from the proposed development to existing and horizon traffic volumes at each specific intersection can be found in Appendix C.

The resulting Synchro 10 simulation reports are provided in Appendix D. Optimal signal timings were used for all traffic scenarios at the signalized intersections of Richmond Street at Keil Drive South and Park Avenue West at Keil Drive South. To quantify the effect of traffic growth on individual intersections within the study area and to assess the need for geometric or traffic infrastructure improvements, the Synchro results were summarized in the following tables:

**Richmond Street at Keil Drive South**

The signalized intersection of Richmond Street at Keil Drive South is currently operating at a very satisfactory level of service. As observed in Tables 1 and 2, it is anticipated that the intersection will continue to operate at a satisfactory level of service in all horizon traffic scenarios, even with the addition of site generated traffic and background traffic growth.

**Table 1: Overall Signalized Intersection Level of Service – Richmond Street at Keil Drive South**

Scenario	Richmond Street at Keil Drive South	
	AM Peak Hour	PM Peak Hour
Existing Traffic	B	B
Existing + Site Generated Traffic	B	B
Total Traffic 2026	B	B
Total Traffic 2031	B	B

**Table 2: Level of Service by Approach – Richmond Street at Keil Drive South**

Scenario	Richmond Street at Keil Drive South							
	AM Peak Hour				PM Peak Hour			
	E/B	W/B	N/B	S/B	E/B	W/B	N/B	S/B
Existing Traffic	B	B	C	B	B	B	C	B
Existing + Site Generated Traffic	B	B	C	B	B	B	C	B
Total Traffic 2026	B	B	C	B	C	B	C	B
Total Traffic 2031	C	B	C	B	B	B	C	C

**Park Avenue West at Keil Drive South**

The signalized intersection of Park Avenue West at Keil Drive South is currently operating at a satisfactory level of service. As observed in Tables 3 and 4, it is anticipated that the intersection will continue to operate at a satisfactory level of service in all horizon traffic scenarios, even with the addition of site generated traffic and background traffic growth.

**Table 3: Overall Signalized Intersection Level of Service – Park Avenue West at Keil Drive South**

Scenario	Park Avenue West at Keil Drive South	
	AM Peak Hour	PM Peak Hour
Existing Traffic	B	B
Existing + Site Generated Traffic	B	B
Total Traffic 2026	B	B
Total Traffic 2031	B	B

**Table 4: Level of Service by Approach – Park Avenue West at Keil Drive South**

Scenario	Park Avenue West at Keil Drive South							
	AM Peak Hour				PM Peak Hour			
	E/B	W/B	N/B	S/B	E/B	W/B	N/B	S/B
Existing Traffic	B	A	C	B	B	A	C	B
Existing + Site Generated	B	A	C	B	B	A	C	B
Total Traffic 2026	B	A	C	B	B	A	C	B
Total Traffic 2031	B	A	C	B	B	A	C	C

**Site Access at Keil Drive South**

The proposed eastbound stop-controlled site access at Keil Drive South was modelled under the assumption that the site will require a single lane for ingress and a single lane for egress. Based on the level of service results provided in Table 5, it is anticipated that the site access will operate favourably in all horizon year scenarios.

**Table 5: Level of Service by Approach – Site Access at Keil Drive South**

Scenario	Site Access at Keil Drive South							
	AM Peak Hour				PM Peak Hour			
	E/B	W/B	N/B	S/B	E/B	W/B	N/B	S/B
Existing + Site Generated	B	-	A	A	B	-	A	A
Total Traffic 2026	B	-	A	A	B	-	A	A
Total Traffic 2031	B	-	A	A	B	-	A	A

**Site Access at Park Avenue West**

The proposed northbound stop-controlled site access at Park Avenue West was modelled under the assumption that the site will require a single lane for ingress and a single lane for egress. Based on the level of service results provided in Table 6, it is anticipated that the site access will operate favourably in all horizon year scenarios.

**Table 6: Level of Service by Approach – Site Access at Park Avenue West**

Scenario	Site Access at Park Avenue West							
	AM Peak Hour				PM Peak Hour			
	E/B	W/B	N/B	S/B	E/B	W/B	N/B	S/B
Existing + Site Generated	A	A	A	-	A	A	B	-
Total Traffic 2026	A	A	A	-	A	A	B	-
Total Traffic 2031	A	A	A	-	A	A	B	-



## SIGHT LINE ANALYSIS

A sight line analysis was completed for the proposed site accesses at Keil Drive South and Park Avenue West. The analysis was completed per the TAC Geometric Design Guide for Canadian Roads (2017). On both Keil Drive South and Park Avenue West, the posted speed limit is 60 km/h, so the analysis was completed for a 70 km/h design speed. As calculated in Appendix E, the minimum intersection sight distance is determined to be 146m for the worst-case left turn egress maneuver and 126m for the right turn egress maneuver.

Based on the sight lines illustrated on Figures 9A and 9B, it is the engineers' opinion that there is sufficient sight distance in both directions for safe egress from the proposed accesses. However, it is the engineers' recommendation that the developer and road authority verify all sight lines on-site to ensure that all boulevard areas within the right-of-way are clear of obstructions before the commencement of construction.

## SUMMARY AND CONCLUSIONS

A residential / commercial development has been proposed for lands situated at 460 Keil Drive South in Chatham, Ontario. The proposed development site is located on the southwest corner of Keil Drive South at Park Avenue West. The study area includes the proposed eastbound stop-controlled site access at Keil Drive South, the northbound stop-controlled site access at Park Avenue West, and the signalized intersections of Richmond Street and Park Avenue West at Keil Drive South.

The proposed site plan is comprised of a total of 117 residential units in a single six-storey apartment building; the apartment building will also include approximately 5005 sq. ft. of ground floor commercial space. A total of 208 parking spaces are proposed to accommodate the entire development.

Using recently obtained turning movement counts and applying the best available trip generation and distribution data and methodologies, an analysis was completed to quantify the potential impact of the development on area traffic operations. Site generated traffic volumes were added to all horizon traffic scenarios; background traffic was increased by 1% per year for the 2026 and 2031 horizon years. Upon completion of the analysis, it was concluded that:

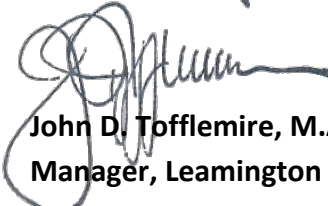
- The signalized intersection of Richmond Street at Keil Drive South is currently operating at a satisfactory level of service; even with the addition of site generated traffic and background traffic growth, it is anticipated that the intersection will continue to operate at a satisfactory level of service in all horizon traffic scenarios;

- The intersection of Park Avenue West at Keil Drive South is currently operating at a satisfactory level of service; even with the addition of site generated traffic and background traffic growth, it is anticipated that the intersection will continue to operate at a satisfactory level of service in all horizon traffic scenarios;
- The proposed site accesses at Keil Drive South and Park Avenue West will operate favourably in all horizon scenarios; there is sufficient sight distance to accommodate safe egress from the proposed site accesses.


Therefore, based on the results of the technical work, it is the engineers' opinion that the proposed development will not adversely affect area traffic operations.

All of which is respectfully submitted,

**RC Spencer Associates Inc.**



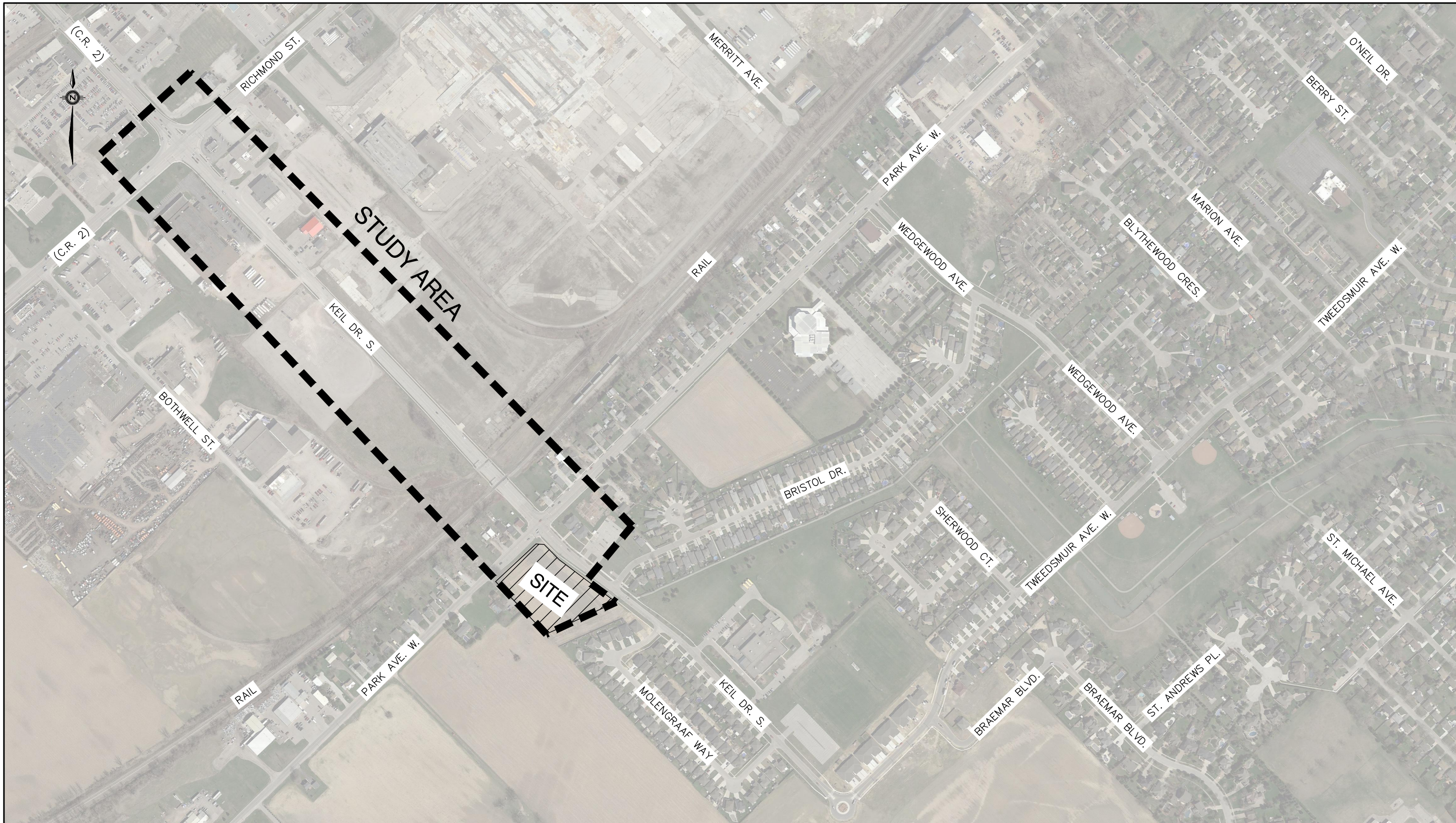
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Associate / Traffic Operations Project Engineer



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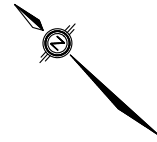
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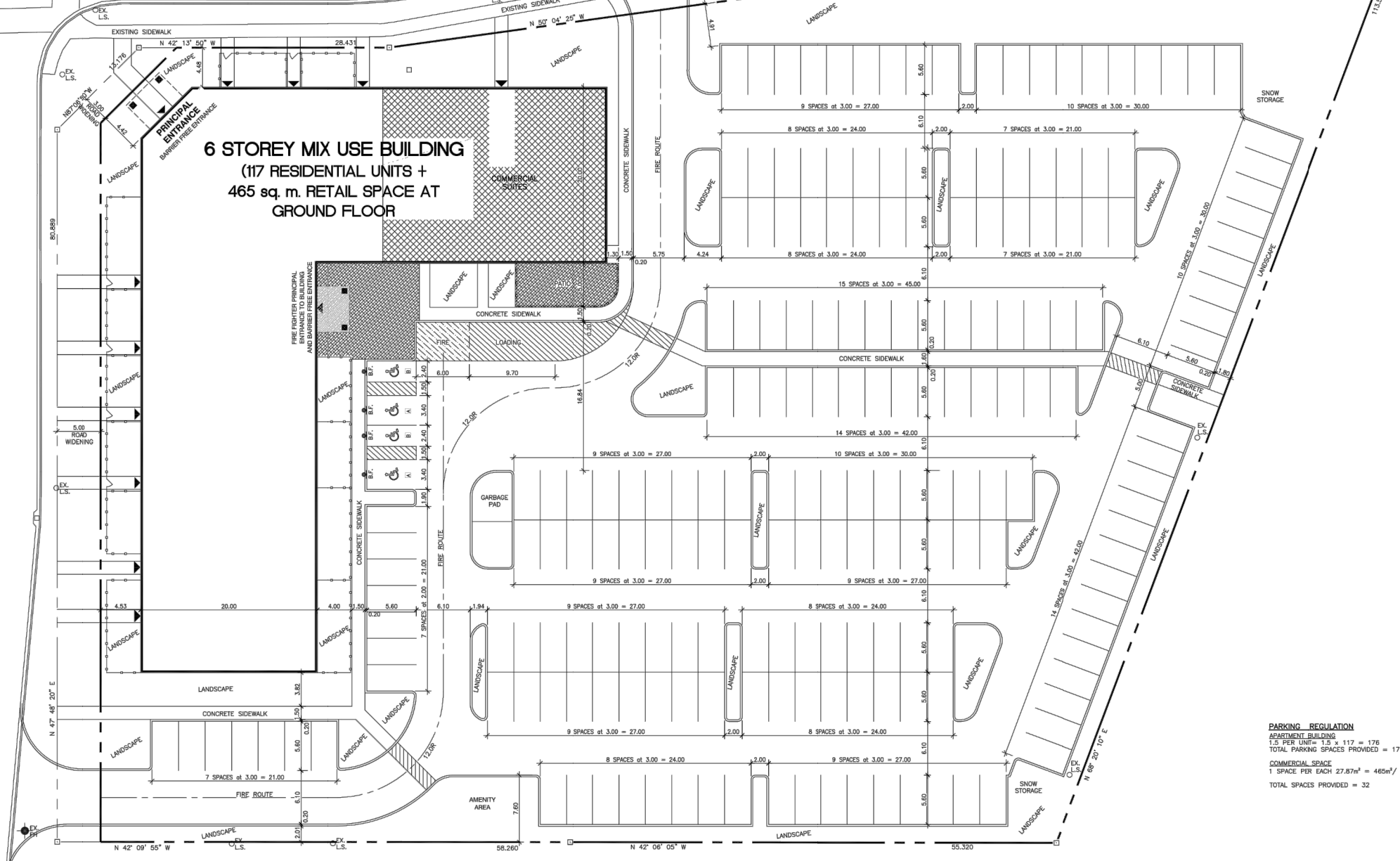
**STUDY AREA**



PARK AVENUE WEST

KEIL DRIVE SOUTH

**6 STOREY MIX USE BUILDING**  
 (117 RESIDENTIAL UNITS +  
 465 sq. m. RETAIL SPACE AT  
 GROUND FLOOR)



**PARKING REGULATION**  
 APARTMENT BUILDING  
 1.5 PER UNIT = 1.5 x 117 = 176  
 TOTAL PARKING SPACES PROVIDED = 176  
 COMMERCIAL SPACE  
 1 SPACE PER EACH 27.87m<sup>2</sup> = 465m<sup>2</sup> / 27.87m<sup>2</sup> = 17 SPACES  
 TOTAL SPACES PROVIDED = 32



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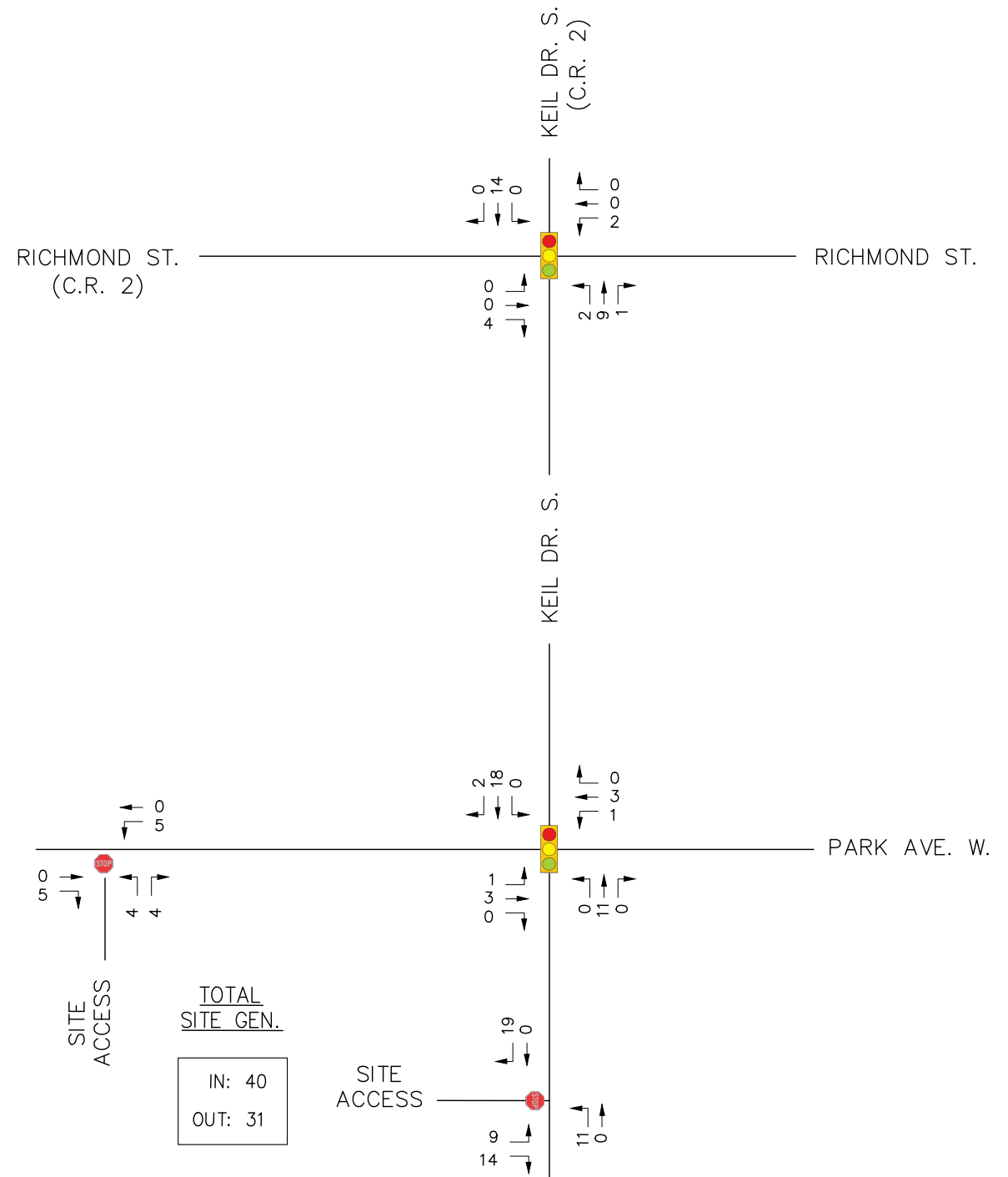
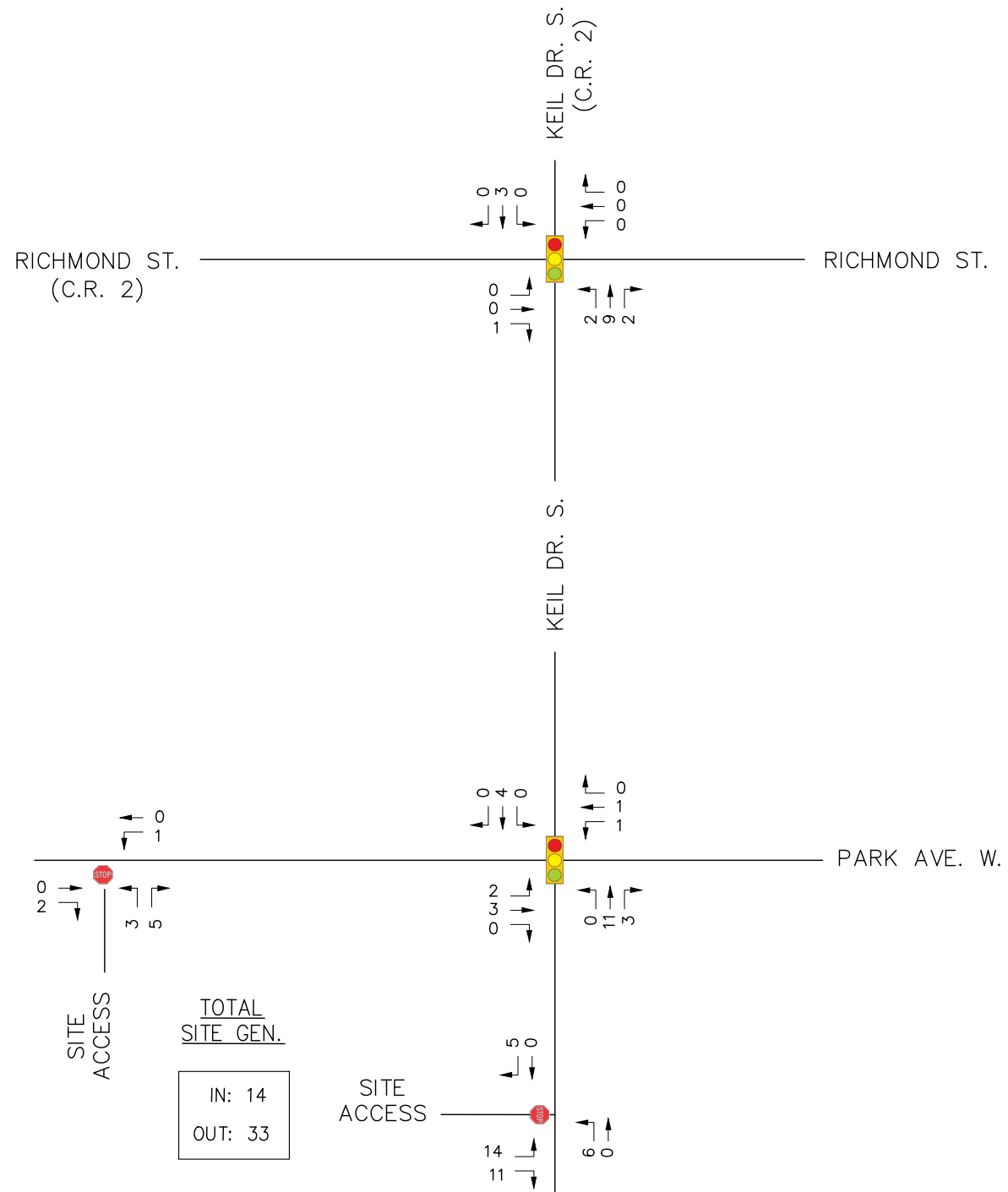
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**SITE PLAN**

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 FIGURE NO.  
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SITE GENERATED TRAFFIC (AM)

SITE GENERATED TRAFFIC (PM)



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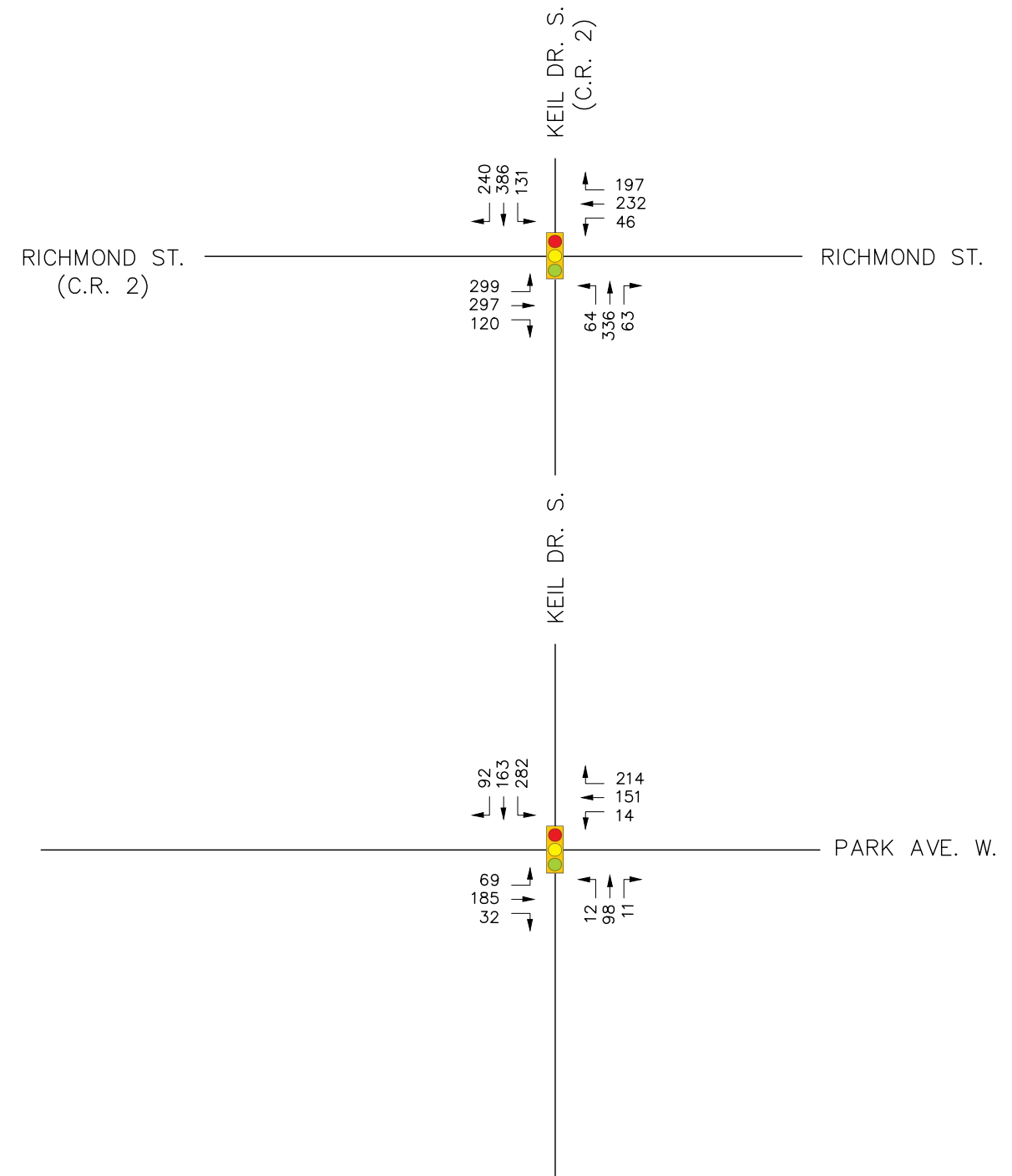
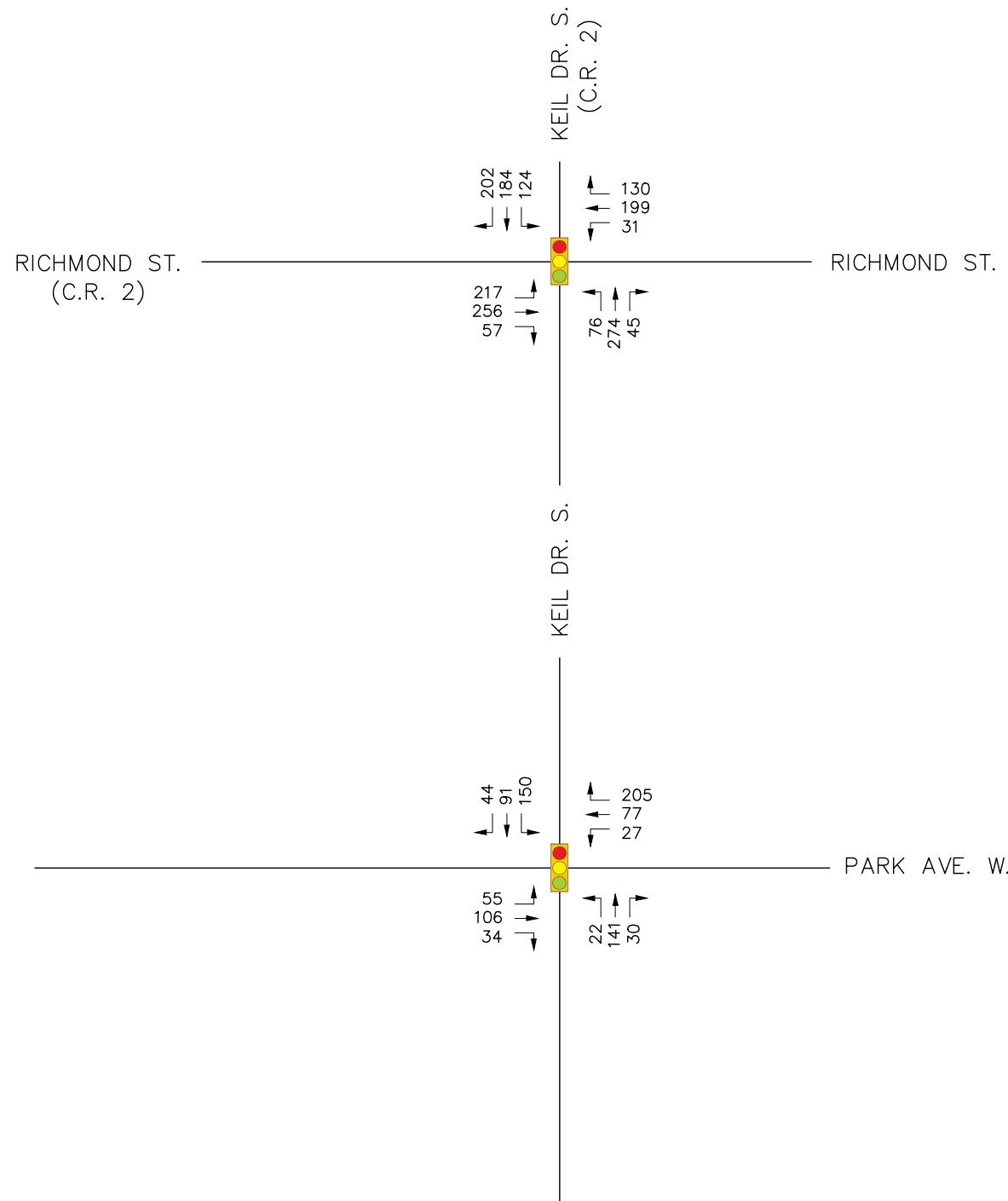
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SITE GENERATED TRAFFIC (AM/PM PEAK HOUR)

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FIGURE NO.	4
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EXISTING TRAFFIC (AM)

EXISTING TRAFFIC (PM)



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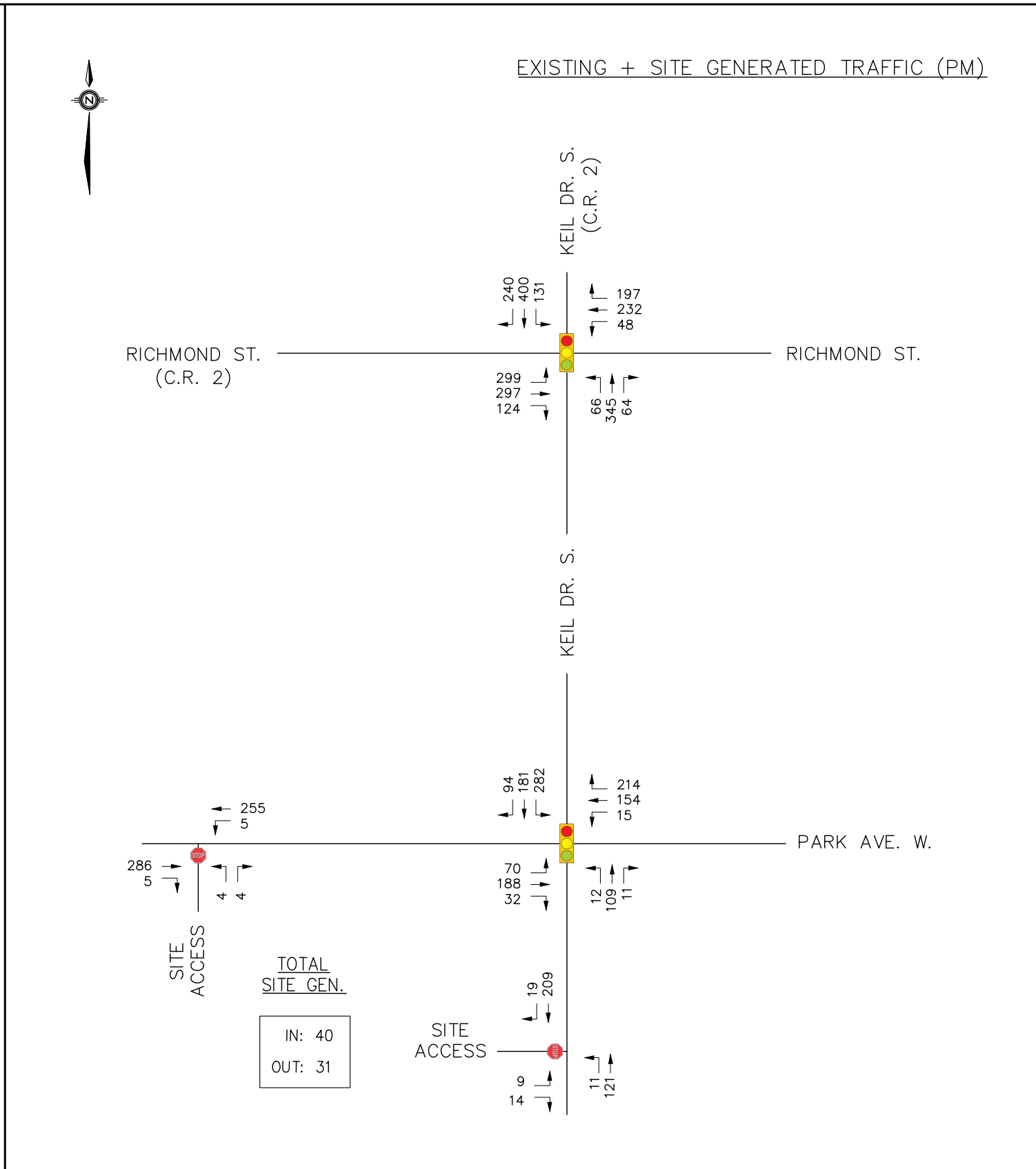
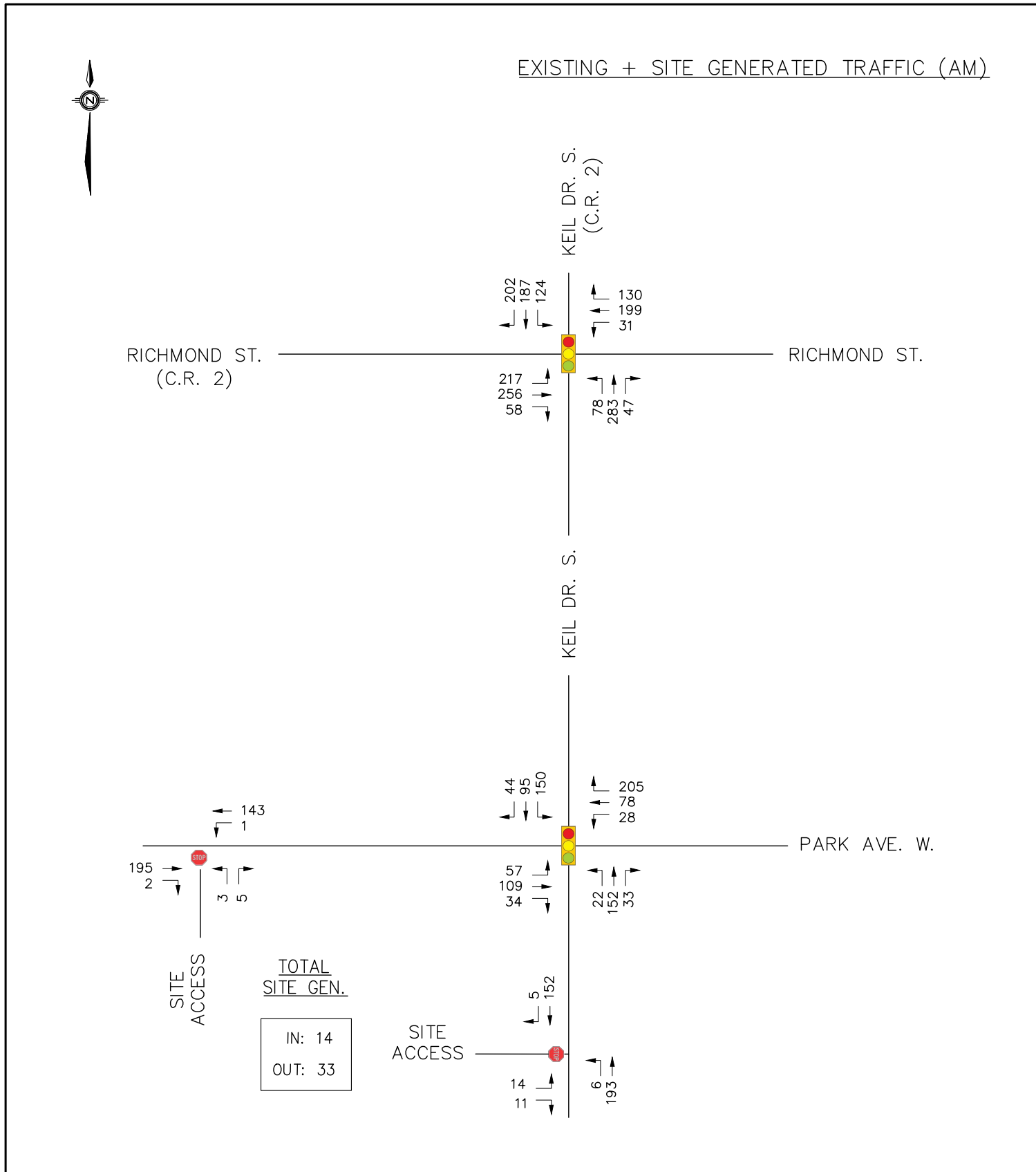
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EXISTING TRAFFIC  
(AM/PM PEAK HOUR)

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FIGURE NO.  
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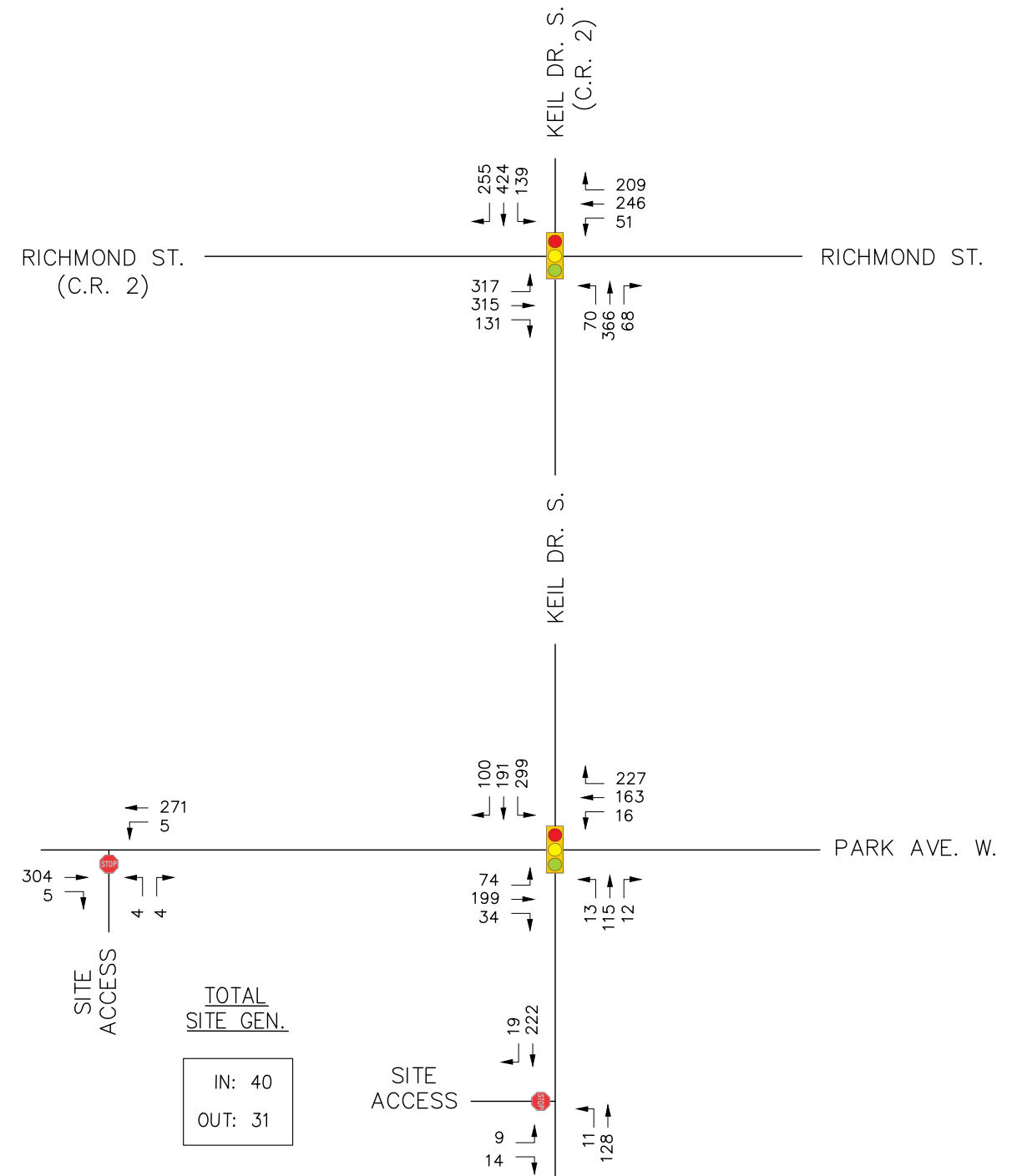
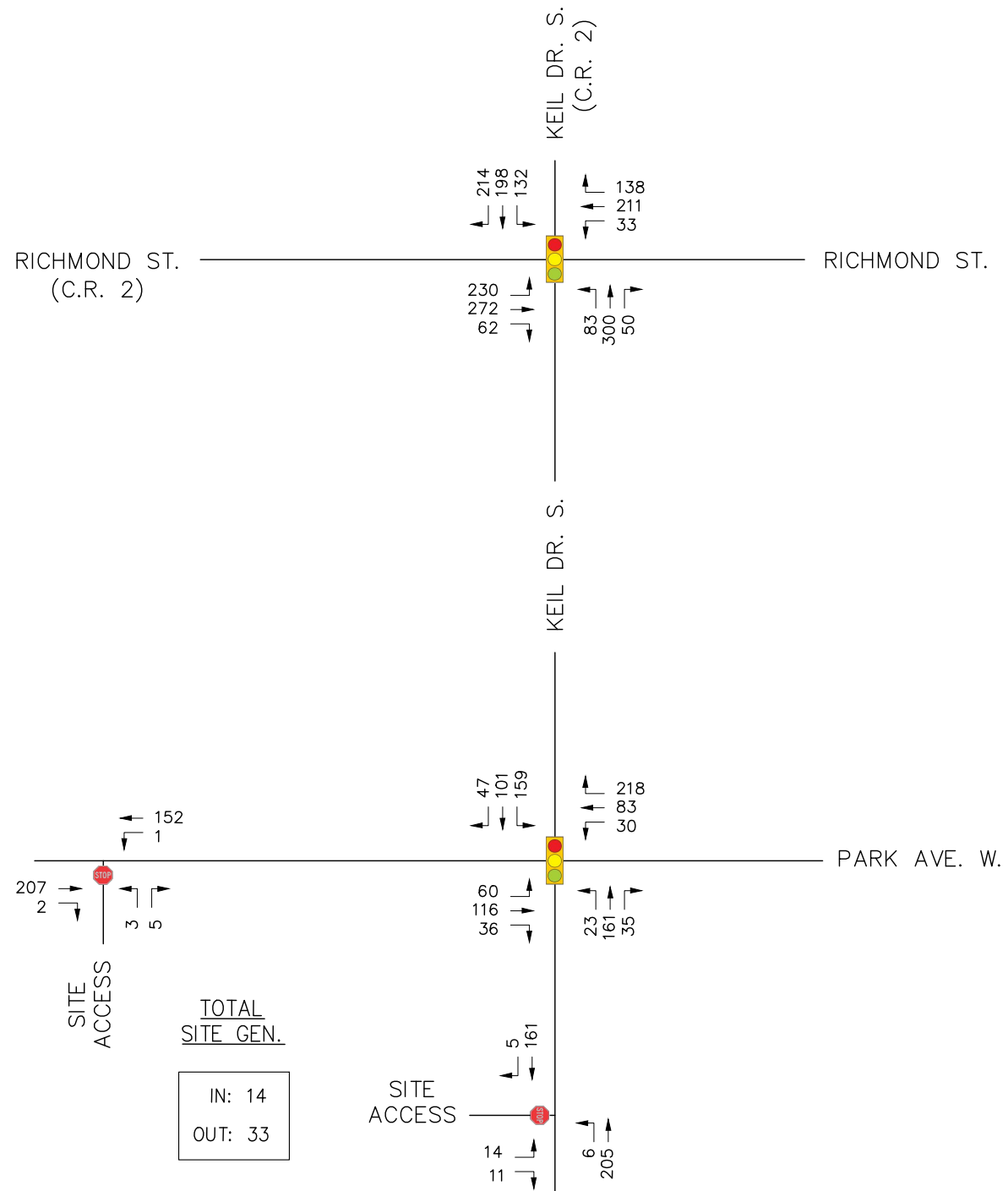


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TOTAL TRAFFIC 2026 (AM)

TOTAL TRAFFIC 2026 (PM)



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TOTAL TRAFFIC 2026  
(AM/PM PEAK HOUR)

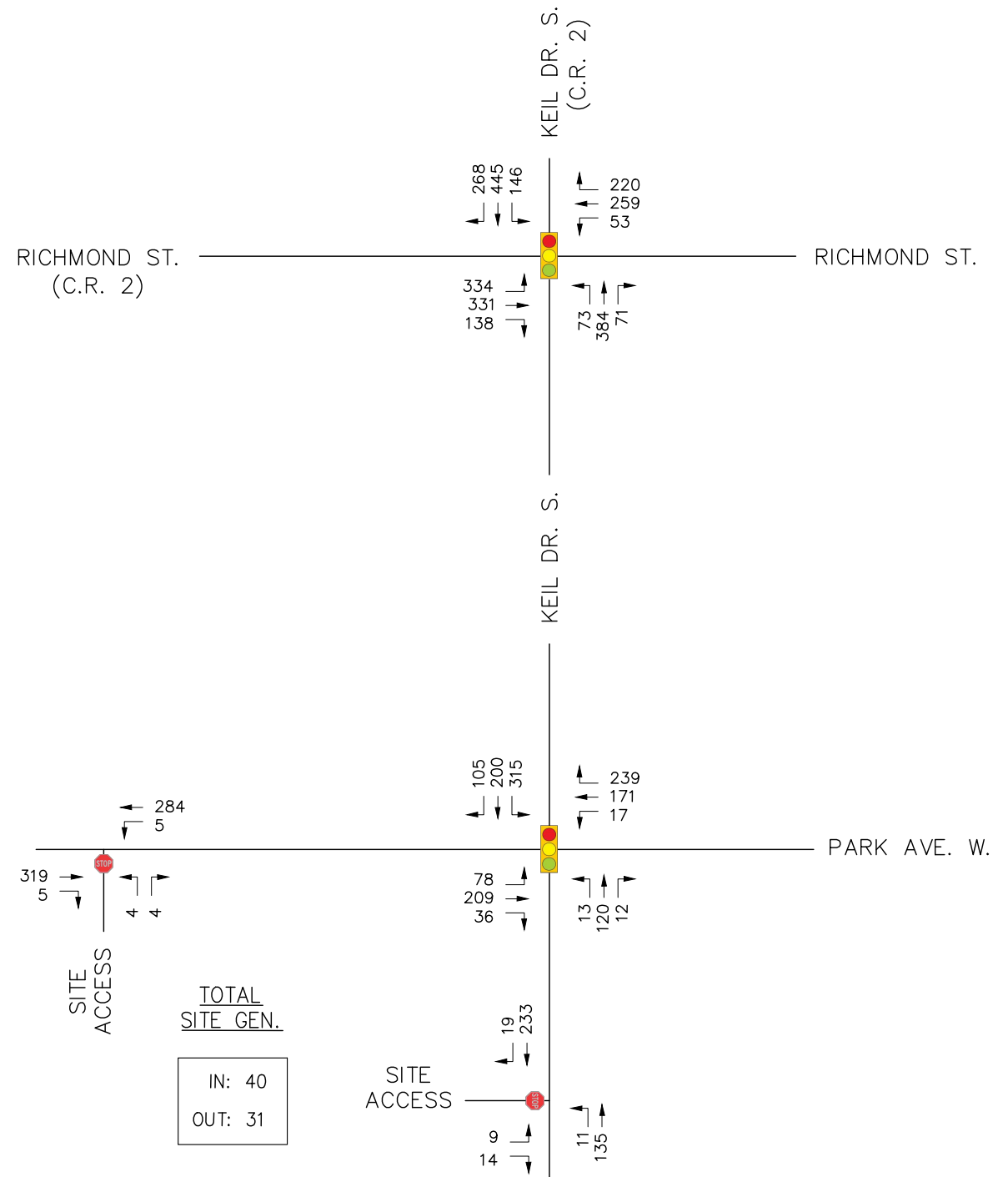
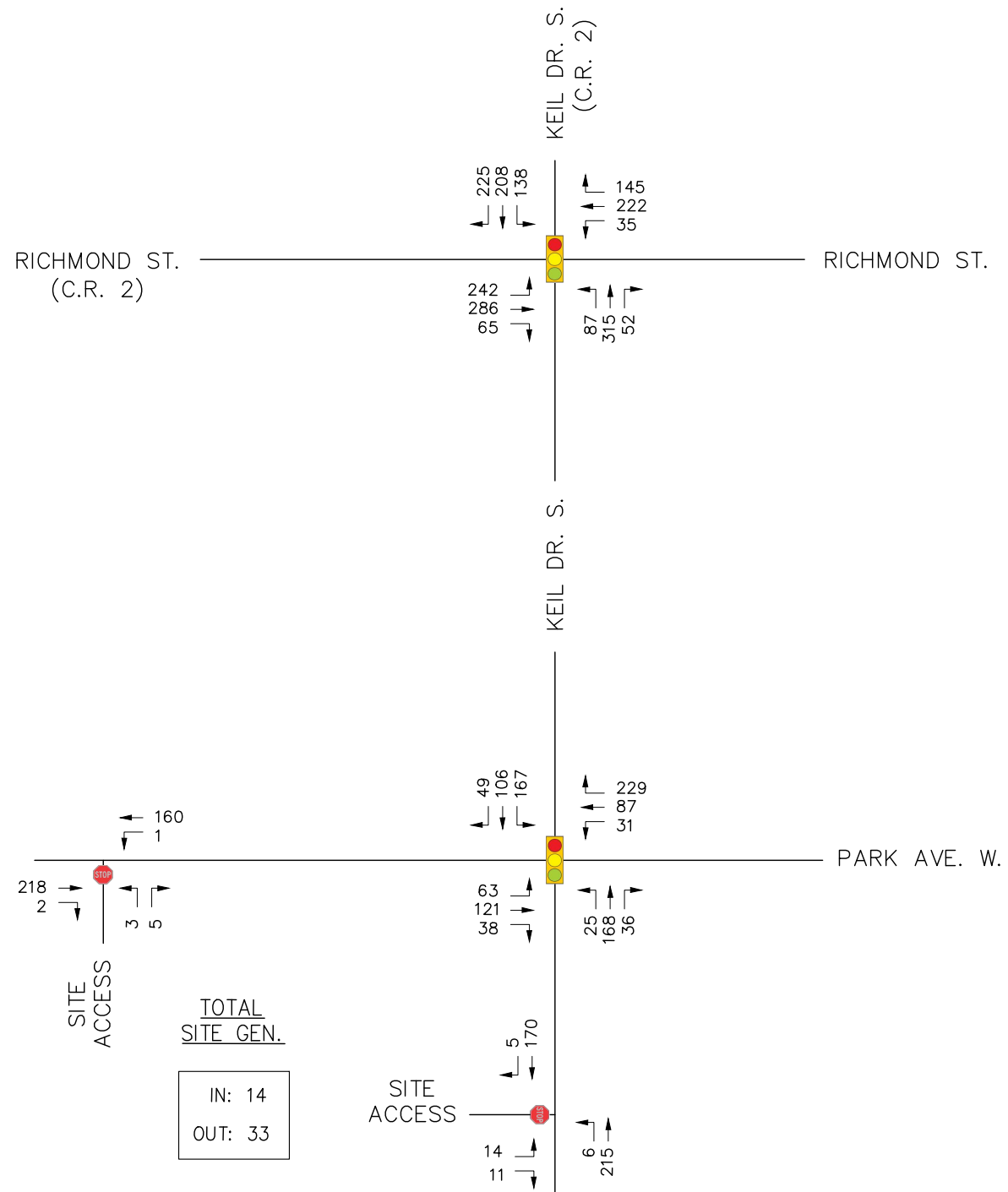
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TOTAL TRAFFIC 2031 (AM)

TOTAL TRAFFIC 2031 (PM)



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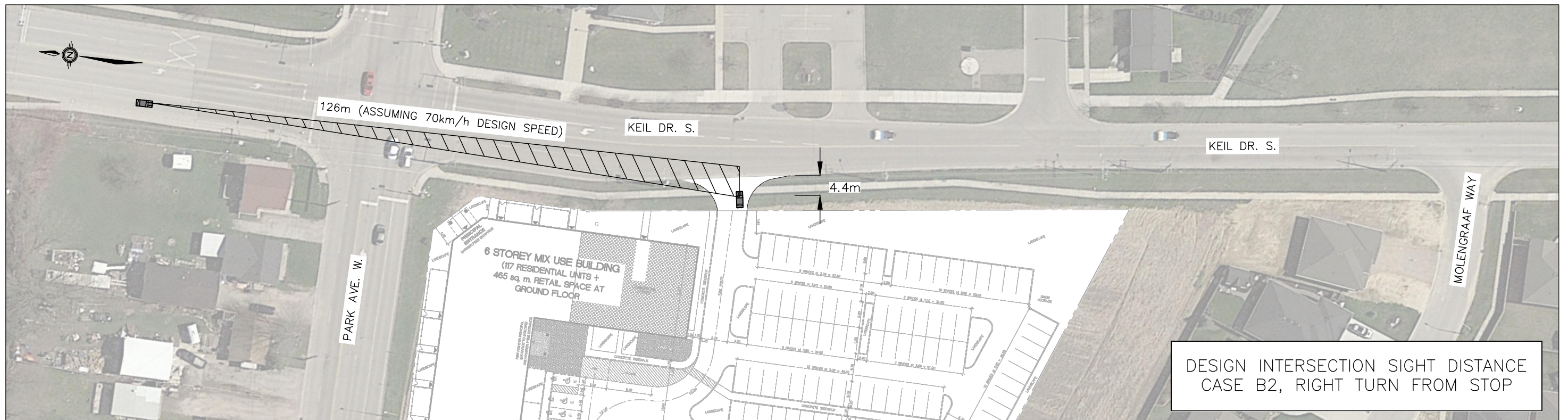
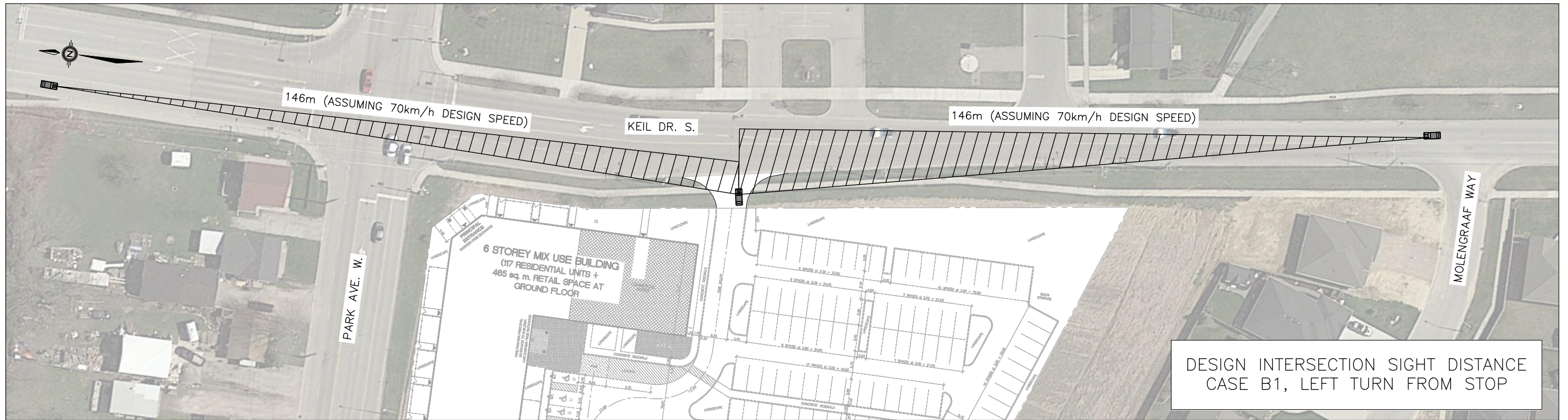


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TOTAL TRAFFIC 2031  
(AM/PM PEAK HOUR)

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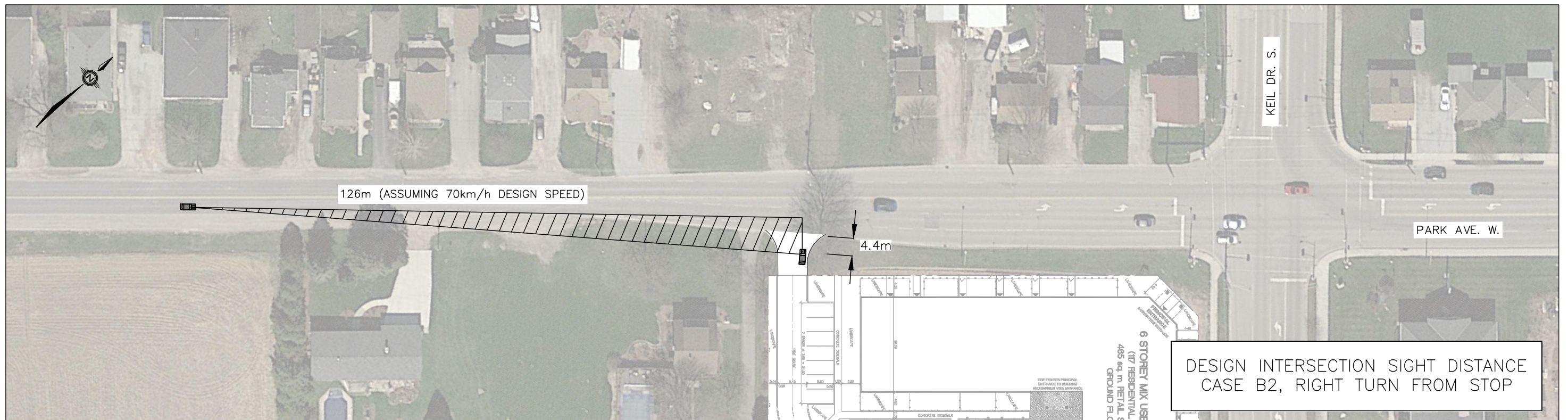
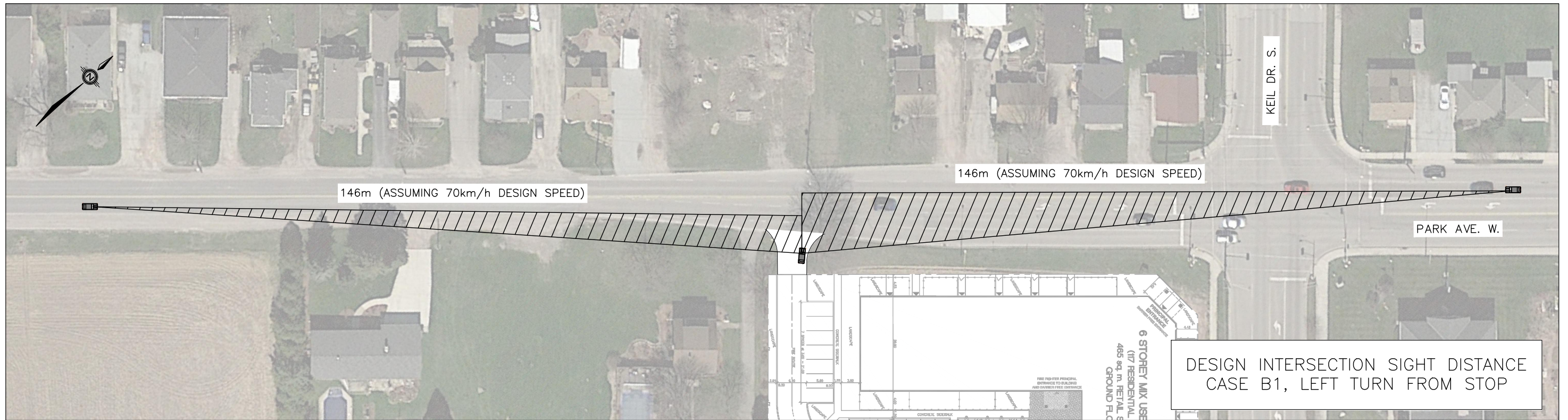
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**SIGHT LINE ANALYSIS  
SITE ACCESS AT  
KEIL DRIVE SOUTH**

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FIGURE NO.	9A
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**SIGHT LINE ANALYSIS  
SITE ACCESS AT  
PARK AVENUE WEST**

PROJECT NO.	20-1083
FIGURE NO.	9B
OF	9

## **Appendix A**

# **TRAFFIC DATA COLLECTION**

**Richmond Street at Keil Drive South  
Park Avenue West at Keil Drive South**

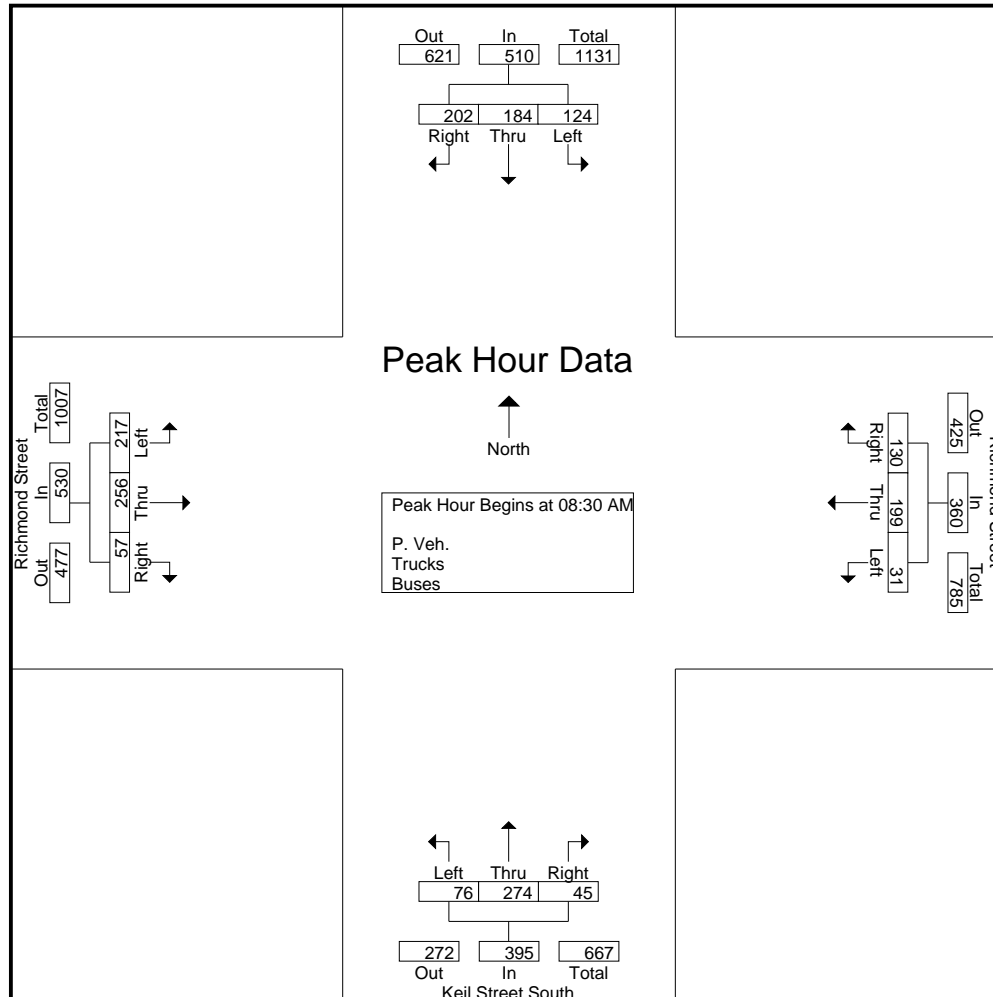
Date: 17 December 2020  
 Counted By: Kevin Brito  
 Weather Conditions: Overcast / Snow  
 Richmond Street at Keil Drive South

Groups Printed- P. Veh. - Trucks - Buses

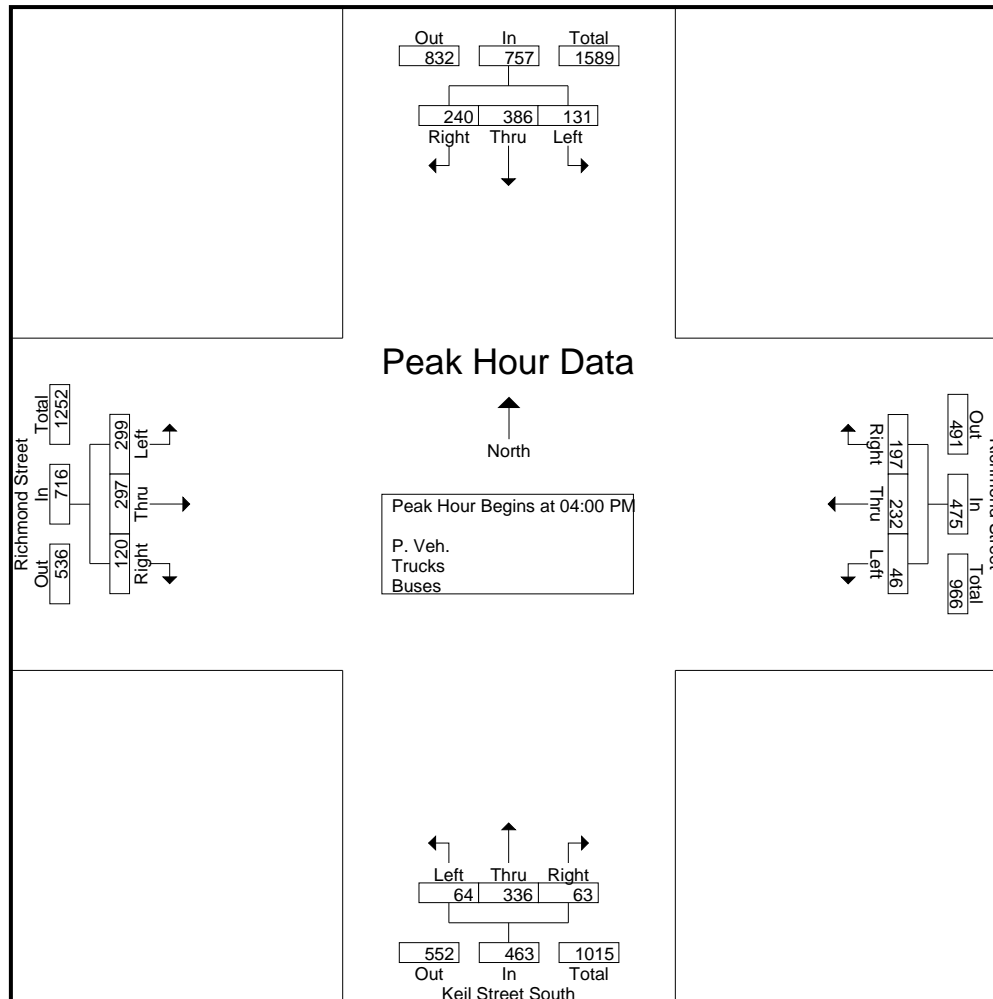
Start Time	Keil Street South N/B					S/B					Richmond Street E/B					Richmond Street W/B					Exclu. Total	Inclu. Total	Int. Total
	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total			
07:30 AM	7	47	35	(2)	89	45	37	16	(0)	98	7	58	44	(0)	109	16	60	4	(0)	80	2	376	378
07:45 AM	19	81	24	(1)	124	45	44	37	(0)	126	9	76	49	(2)	134	17	49	15	(1)	81	4	465	469
Total	26	128	59	(3)	213	90	81	53	(0)	224	16	134	93	(2)	243	33	109	19	(1)	161	6	841	847
08:00 AM	14	52	26	(1)	92	45	49	15	(0)	109	20	70	55	(0)	145	30	67	3	(0)	100	1	446	447
08:15 AM	11	49	21	(0)	81	46	48	18	(0)	112	21	66	51	(0)	138	20	42	13	(1)	75	1	406	407
08:30 AM	17	54	24	(0)	95	48	42	18	(1)	108	15	87	73	(0)	175	26	46	10	(0)	82	1	460	461
08:45 AM	10	86	27	(0)	123	53	46	43	(0)	142	15	54	48	(0)	117	33	54	8	(0)	95	0	477	477
Total	52	241	98	(1)	391	192	185	94	(1)	471	71	277	227	(0)	575	109	209	34	(1)	352	3	1789	1792
09:00 AM	14	72	17	(1)	103	51	49	37	(0)	137	14	56	45	(0)	115	32	51	6	(0)	89	1	444	445
09:15 AM	4	62	8	(0)	74	50	47	26	(0)	123	13	59	51	(1)	123	39	48	7	(0)	94	1	414	415
09:30 AM	4	43	17	(1)	64	39	43	38	(0)	120	15	60	54	(0)	129	32	59	2	(0)	93	1	406	407
09:45 AM	9	51	22	(0)	82	53	45	27	(0)	125	25	45	47	(1)	117	26	61	9	(3)	96	4	420	424
Total	31	228	64	(2)	323	193	184	128	(0)	505	67	220	197	(2)	484	129	219	24	(3)	372	7	1684	1691
*** BREAK ***																							
04:00 PM	11	84	15	(0)	110	61	97	29	(1)	187	51	80	79	(0)	210	55	61	11	(2)	127	3	634	637
04:15 PM	17	84	17	(1)	118	59	107	35	(0)	201	24	72	61	(0)	157	44	63	11	(1)	118	2	594	596
04:30 PM	16	76	19	(2)	111	58	86	38	(0)	182	26	80	75	(1)	181	50	60	11	(0)	121	3	595	598
04:45 PM	19	92	13	(0)	124	62	96	29	(0)	187	19	65	84	(1)	168	48	48	13	(0)	109	1	588	589
Total	63	336	64	(3)	463	240	386	131	(1)	757	120	297	299	(2)	716	197	232	46	(3)	475	9	2411	2420
05:00 PM	12	74	18	(1)	104	56	109	39	(0)	204	31	71	88	(2)	190	39	64	20	(0)	123	3	621	624
05:15 PM	14	70	14	(0)	98	62	99	27	(0)	188	34	53	68	(3)	155	55	56	12	(1)	123	4	564	568
05:30 PM	11	77	23	(2)	111	42	78	35	(0)	155	25	40	46	(1)	111	17	35	7	(1)	59	4	436	440
05:45 PM	6	53	13	(0)	72	52	64	32	(0)	148	15	52	49	(0)	116	26	41	8	(0)	75	0	411	411
Total	43	274	68	(3)	385	212	350	133	(0)	695	105	216	251	(6)	572	137	196	47	(2)	380	11	2032	2043
Grand Total	215	1207	353	(12)	1775	927	1186	539	(2)	2652	379	1144	1067	(12)	2590	605	965	170	(10)	1740	36	8757	8793
Apprch %	12.1	68	19.9			35	44.7	20.3			14.6	44.2	41.2			34.8	55.5	9.8					
Total %	2.5	13.8	4		20.3	10.6	13.5	6.2		30.3	4.3	13.1	12.2		29.6	6.9	11	1.9		19.9	0.4	99.6	
P. Veh.	207	1183	334		1736	866	1175	510		2553	353	1065	1009		2439	588	894	167		1659	0	0	8387
% P. Veh.	96.3	98	94.6	100	97.1	93.4	99.1	94.6	100	96.2	93.1	93.1	94.6	100	93.7	97.2	92.6	98.2	100	94.8	0	0	95.4
Trucks	3	17	17		37	46	7	8		61	24	67	49		140	13	67	3		83	0	0	321
% Trucks	1.4	1.4	4.8	0	2.1	5	0.6	1.5	0	2.3	6.3	5.9	4.6	0	5.4	2.1	6.9	1.8	0	4.7	0	0	3.7
Buses	5	7	2		14	15	4	21		40	2	12	9		23	4	4	0		8	0	0	85
% Buses	2.3	0.6	0.6	0	0.8	1.6	0.3	3.9	0	1.5	0.5	1	0.8	0	0.9	0.7	0.4	0	0	0.5	0	0	1



Start Time	Keil Street South N/B				S/B				Richmond Street E/B				Richmond Street W/B				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 07:30 AM to 11:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 08:30 AM																	
08:30 AM	17	54	24	95	48	42	18	108	15	87	73	175	26	46	10	82	460
08:45 AM	10	86	27	123	53	46	43	142	15	54	48	117	33	54	8	95	477
09:00 AM	14	72	17	103	51	49	37	137	14	56	45	115	32	51	6	89	444
09:15 AM	4	62	8	74	50	47	26	123	13	59	51	123	39	48	7	94	414
Total Volume	45	274	76	395	202	184	124	510	57	256	217	530	130	199	31	360	1795
% App. Total	11.4	69.4	19.2		39.6	36.1	24.3		10.8	48.3	40.9		36.1	55.3	8.6		
PHF	.662	.797	.704	.803	.953	.939	.721	.898	.950	.736	.743	.757	.833	.921	.775	.947	.941



Start Time	Keil Street South N/B				S/B				Richmond Street E/B				Richmond Street W/B				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 12:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:00 PM																	
04:00 PM	11	84	15	110	61	97	29	187	51	80	79	210	55	61	11	127	634
04:15 PM	17	84	17	118	59	107	35	201	24	72	61	157	44	63	11	118	594
04:30 PM	16	76	19	111	58	86	38	182	26	80	75	181	50	60	11	121	595
04:45 PM	19	92	13	124	62	96	29	187	19	65	84	168	48	48	13	109	588
Total Volume	63	336	64	463	240	386	131	757	120	297	299	716	197	232	46	475	2411
% App. Total	13.6	72.6	13.8		31.7	51	17.3		16.8	41.5	41.8		41.5	48.8	9.7		
PHF	.829	.913	.842	.933	.968	.902	.862	.942	.588	.928	.890	.852	.895	.921	.885	.935	.951





Date: 16 December 2020  
 Counted By: Kevin Brito  
 Weather Conditions: Overcast / Snow  
 Park Avenue West at Keil Drive South

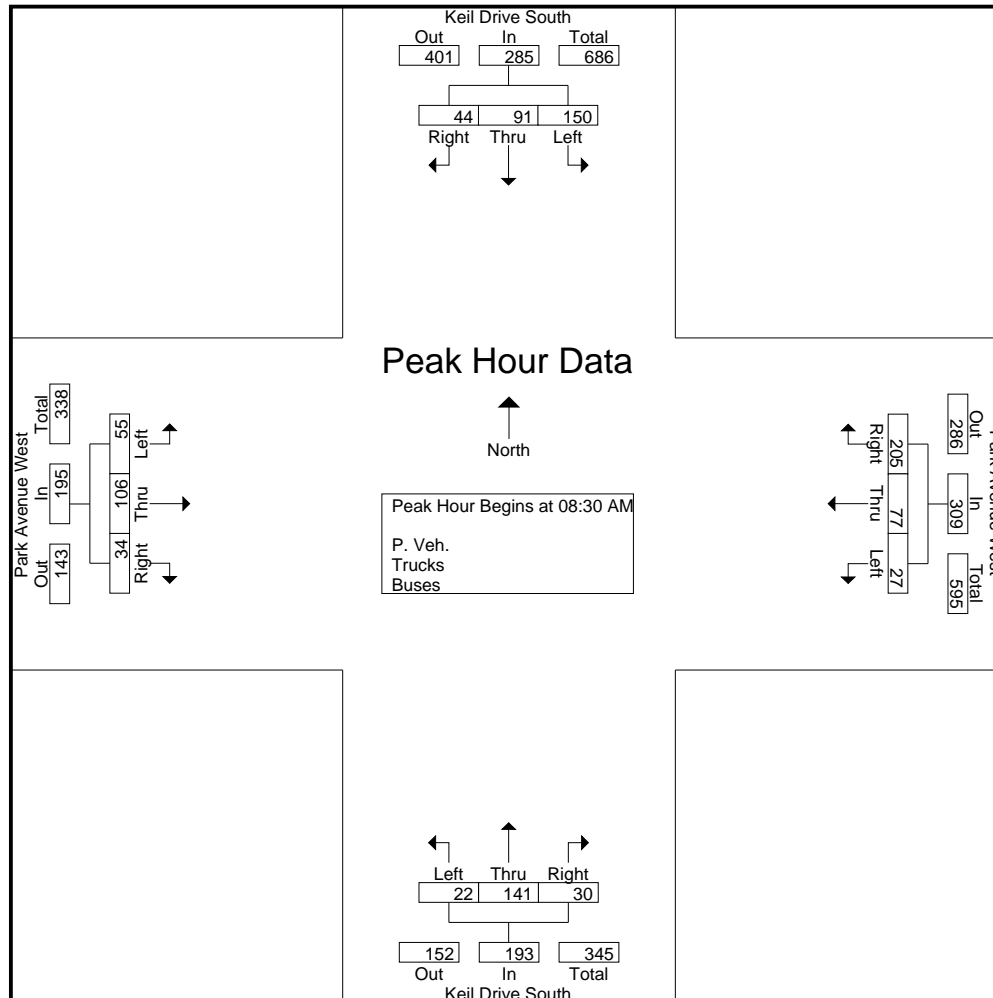
Groups Printed- P. Veh. - Trucks - Buses

Start Time	Keil Drive South N/B					Keil Drive South S/B					Park Avenue West E/B					Park Avenue West W/B					Exclu. Total	Inclu. Total	Int. Total
	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total			
07:30 AM	4	33	5	(0)	42	8	10	26	(0)	44	1	27	18	(0)	46	50	35	7	(0)	92	0	224	224
07:45 AM	1	32	6	(0)	39	13	10	26	(0)	49	10	42	22	(0)	74	51	30	0	(0)	81	0	243	243
Total	5	65	11	(0)	81	21	20	52	(0)	93	11	69	40	(0)	120	101	65	7	(0)	173	0	467	467
08:00 AM	2	28	5	(0)	35	11	22	27	(0)	60	3	25	15	(0)	43	38	36	3	(0)	77	0	215	215
08:15 AM	1	28	1	(0)	30	8	24	22	(0)	54	8	32	14	(0)	54	39	20	10	(0)	69	0	207	207
08:30 AM	15	53	9	(0)	77	10	23	28	(0)	61	16	29	19	(0)	64	37	19	17	(0)	73	0	275	275
08:45 AM	8	52	6	(0)	66	14	23	28	(0)	65	11	28	14	(0)	53	65	27	6	(0)	98	0	282	282
Total	26	161	21	(0)	208	43	92	105	(0)	240	38	114	62	(0)	214	179	102	36	(0)	317	0	979	979
09:00 AM	3	19	4	(0)	26	8	21	51	(0)	80	2	18	10	(0)	30	51	16	3	(0)	70	0	206	206
09:15 AM	4	17	3	(0)	24	12	24	43	(0)	79	5	31	12	(0)	48	52	15	1	(1)	68	1	219	220
09:30 AM	5	17	4	(0)	26	10	20	43	(0)	73	4	25	12	(0)	41	43	14	0	(0)	57	0	197	197
09:45 AM	2	28	5	(0)	35	7	14	29	(1)	50	4	21	15	(1)	40	57	11	1	(0)	69	2	194	196
Total	14	81	16	(0)	111	37	79	166	(1)	282	15	95	49	(1)	159	203	56	5	(1)	264	3	816	819
*** BREAK ***																							
04:00 PM	3	27	7	(1)	37	20	46	60	(0)	126	9	46	19	(0)	74	68	29	2	(0)	99	1	336	337
04:15 PM	4	22	1	(0)	27	32	36	59	(0)	127	7	39	16	(0)	62	52	36	6	(0)	94	0	310	310
04:30 PM	4	29	2	(0)	35	9	43	76	(0)	128	11	47	19	(0)	77	62	42	3	(1)	107	1	347	348
04:45 PM	3	28	3	(0)	34	23	38	75	(0)	136	7	43	14	(0)	64	54	36	3	(0)	93	0	327	327
Total	14	106	13	(1)	133	84	163	270	(0)	517	34	175	68	(0)	277	236	143	14	(1)	393	2	1320	1322
05:00 PM	0	19	6	(0)	25	28	46	72	(0)	146	7	56	20	(0)	83	46	37	2	(0)	85	0	339	339
05:15 PM	3	28	0	(0)	31	25	28	50	(0)	103	8	42	13	(1)	63	55	30	2	(2)	87	3	284	287
05:30 PM	1	11	3	(0)	15	11	33	49	(2)	93	4	26	8	(0)	38	40	19	4	(0)	63	2	209	211
05:45 PM	3	22	2	(0)	27	13	29	29	(0)	71	2	33	8	(0)	43	35	32	4	(0)	71	0	212	212
Total	7	80	11	(0)	98	77	136	200	(2)	413	21	157	49	(1)	227	176	118	12	(2)	306	5	1044	1049
Grand Total	66	493	72	(1)	631	262	490	793	(3)	1545	119	610	268	(2)	997	895	484	74	(4)	1453	10	4626	4636
Apprch %	10.5	78.1	11.4			17	31.7	51.3			11.9	61.2	26.9			61.6	33.3	5.1					
Total %	1.4	10.7	1.6		13.6	5.7	10.6	17.1		33.4	2.6	13.2	5.8		21.6	19.3	10.5	1.6		31.4	0.2	99.8	
P. Veh.	65	488	70		624	253	486	755		1497	106	577	259		944	861	460	70		1394	0	0	4459
% P. Veh.	98.5	99	97.2	100	98.7	96.6	99.2	95.2	100	96.7	89.1	94.6	96.6	100	94.5	96.2	95	94.6	75	95.7	0	0	96.2
Trucks	0	2	1		3	9	1	35		45	0	26	5		31	32	19	2		54	0	0	133
% Trucks	0	0.4	1.4	0	0.5	3.4	0.2	4.4	0	2.9	0	4.3	1.9	0	3.1	3.6	3.9	2.7	25	3.7	0	0	2.9
Buses	1	3	1		5	0	3	3		6	13	7	4		24	2	5	2		9	0	0	44
% Buses	1.5	0.6	1.4	0	0.8	0	0.6	0.4	0	0.4	10.9	1.1	1.5	0	2.4	0.2	1	2.7	0	0.6	0	0	0.9

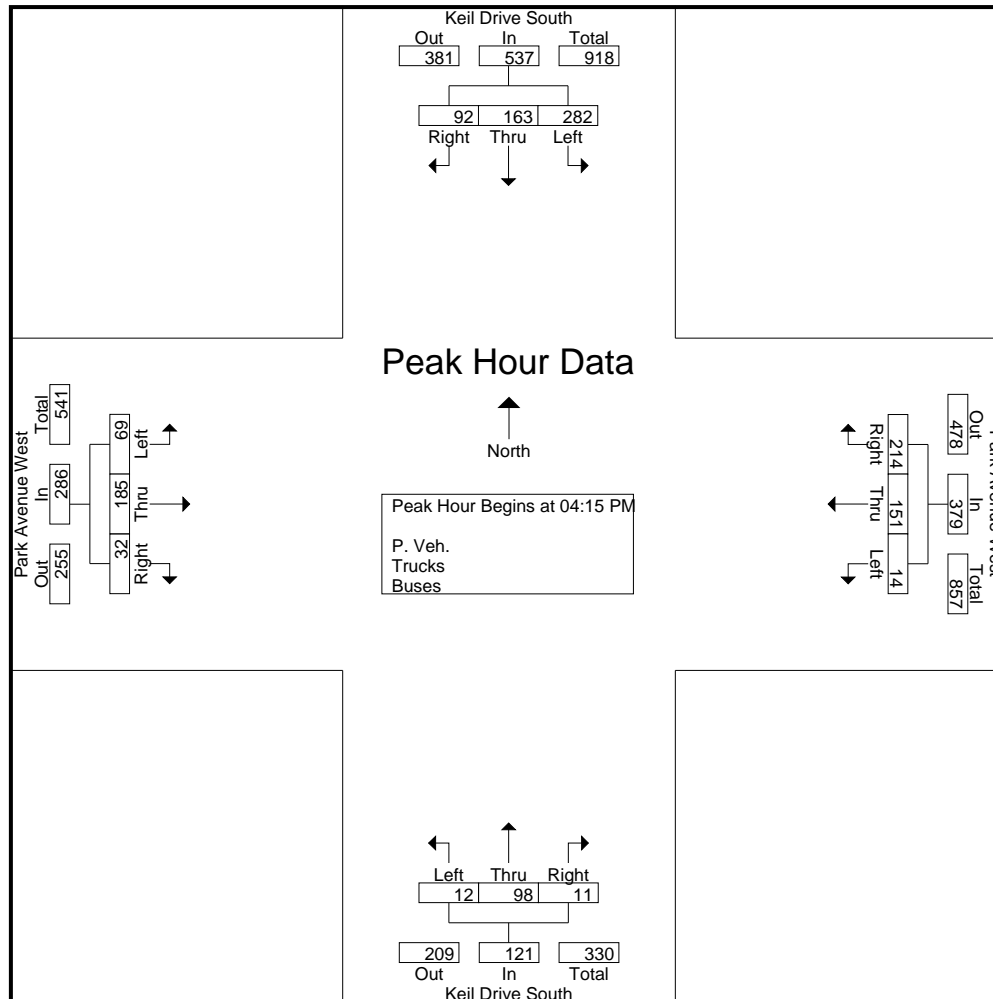


Start Time	Keil Drive South N/B				Keil Drive South S/B				Park Avenue West E/B				Park Avenue West W/B				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
08:30 AM	15	53	9	77	10	23	28	61	16	29	19	64	37	19	17	73	275
08:45 AM	8	52	6	66	14	23	28	65	11	28	14	53	65	27	6	98	282
09:00 AM	3	19	4	26	8	21	51	80	2	18	10	30	51	16	3	70	206
09:15 AM	4	17	3	24	12	24	43	79	5	31	12	48	52	15	1	68	219
Total Volume	30	141	22	193	44	91	150	285	34	106	55	195	205	77	27	309	982
% App. Total	15.5	73.1	11.4		15.4	31.9	52.6		17.4	54.4	28.2		66.3	24.9	8.7		
PHF	.500	.665	.611	.627	.786	.948	.735	.891	.531	.855	.724	.762	.788	.713	.397	.788	.871

Peak Hour Analysis From 07:30 AM to 11:45 AM - Peak 1 of 1  
Peak Hour for Entire Intersection Begins at 08:30 AM



Start Time	Keil Drive South N/B				Keil Drive South S/B				Park Avenue West E/B				Park Avenue West W/B				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 12:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:15 PM																	
04:15 PM	4	22	1	27	32	36	59	127	7	39	16	62	52	36	6	94	310
04:30 PM	4	29	2	35	9	43	76	128	11	47	19	77	62	42	3	107	347
04:45 PM	3	28	3	34	23	38	75	136	7	43	14	64	54	36	3	93	327
05:00 PM	0	19	6	25	28	46	72	146	7	56	20	83	46	37	2	85	339
Total Volume	11	98	12	121	92	163	282	537	32	185	69	286	214	151	14	379	1323
% App. Total	9.1	81	9.9		17.1	30.4	52.5		11.2	64.7	24.1		56.5	39.8	3.7		
PHF	.688	.845	.500	.864	.719	.886	.928	.920	.727	.826	.863	.861	.863	.899	.583	.886	.953



## **Appendix B**

# **ITE TRIP GENERATION MANUAL – 10<sup>TH</sup> EDITION REFERENCES**

# Multifamily Housing (Mid-Rise) (221)

**Vehicle Trip Ends vs: Dwelling Units**  
**On a: Weekday,**  
**Peak Hour of Adjacent Street Traffic,**  
**One Hour Between 7 and 9 a.m.**

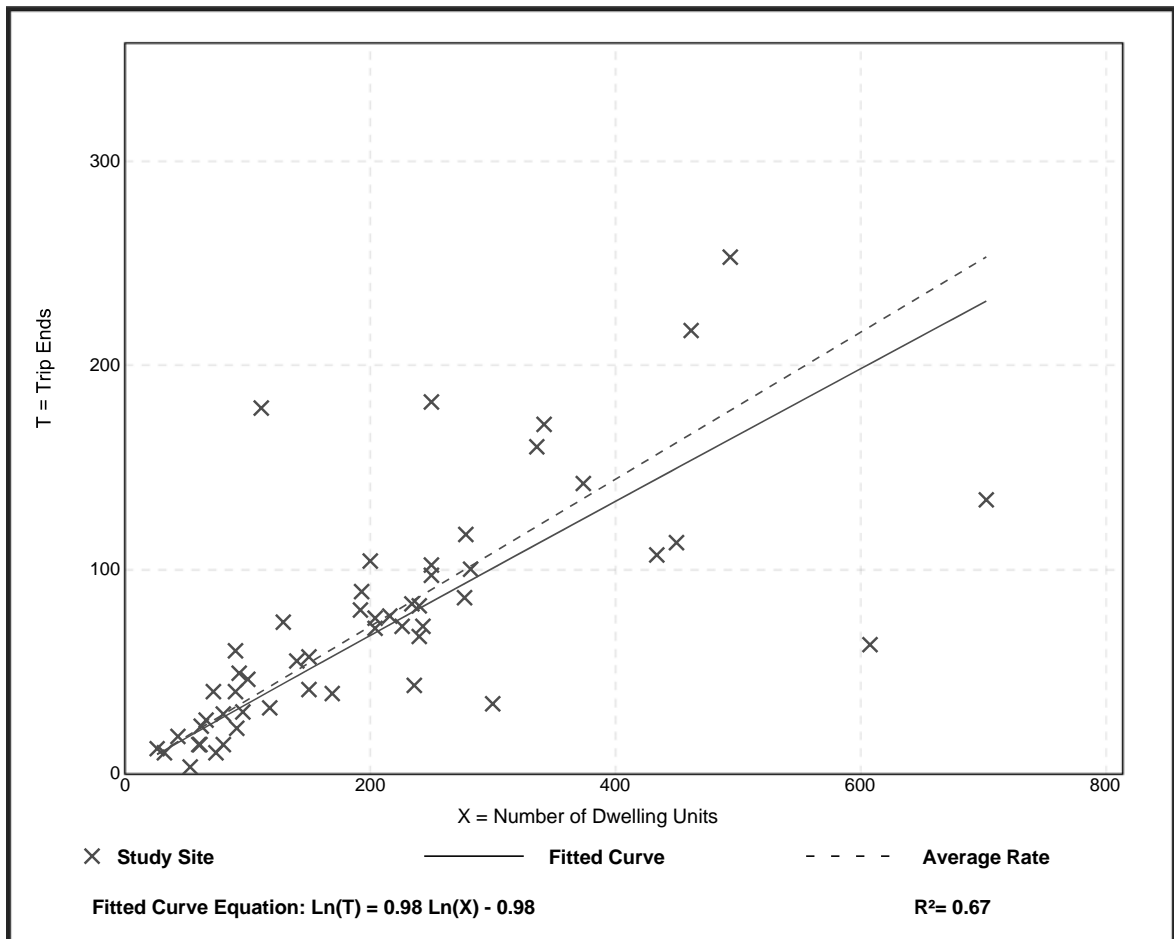
**Setting/Location: General Urban/Suburban**

Number of Studies: 53  
 Avg. Num. of Dwelling Units: 207  
 Directional Distribution: 26% entering, 74% exiting

## Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.36	0.06 - 1.61	0.19

## Data Plot and Equation



# Multifamily Housing (Mid-Rise) (221)

**Vehicle Trip Ends vs: Dwelling Units**  
**On a: Weekday,**  
**Peak Hour of Adjacent Street Traffic,**  
**One Hour Between 4 and 6 p.m.**

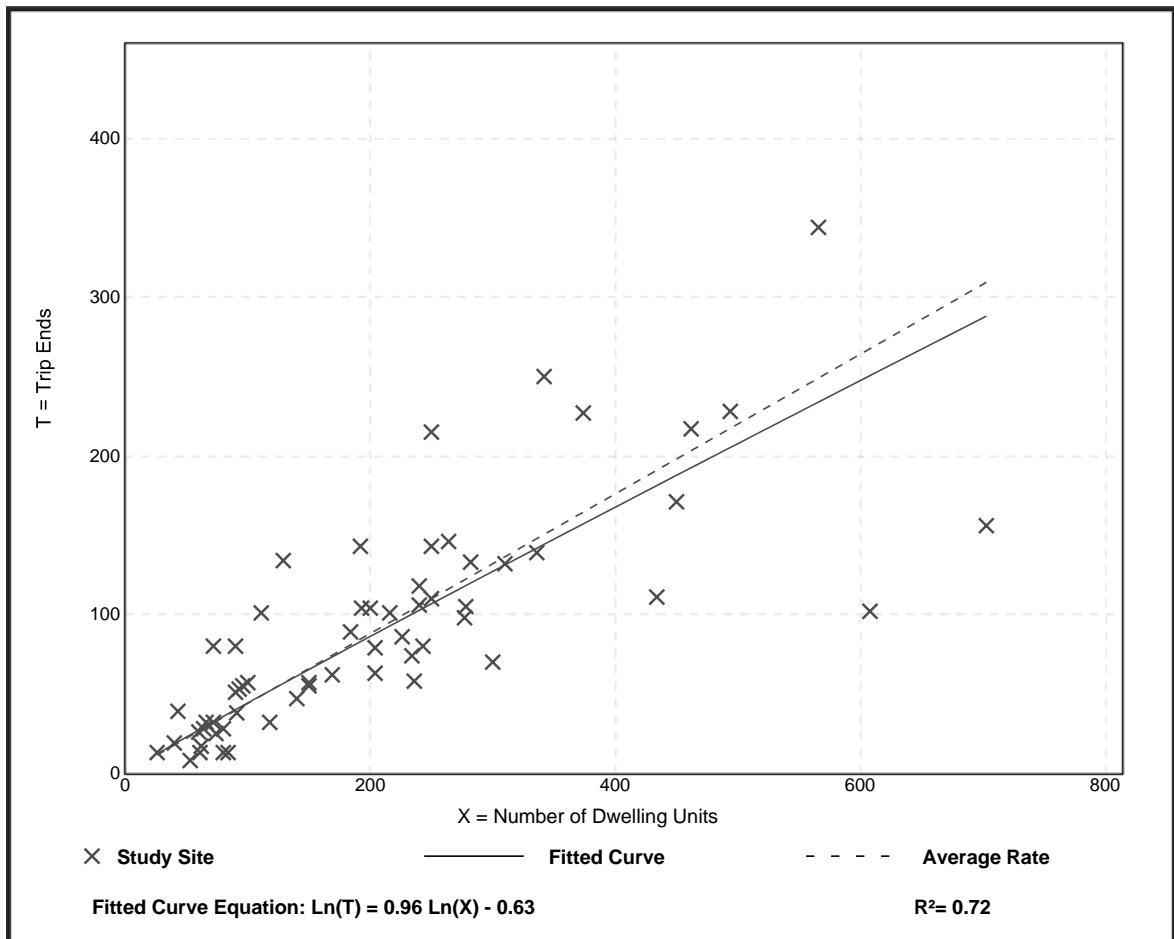
**Setting/Location: General Urban/Suburban**

Number of Studies: 60  
 Avg. Num. of Dwelling Units: 208  
 Directional Distribution: 61% entering, 39% exiting

## Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.44	0.15 - 1.11	0.19

## Data Plot and Equation



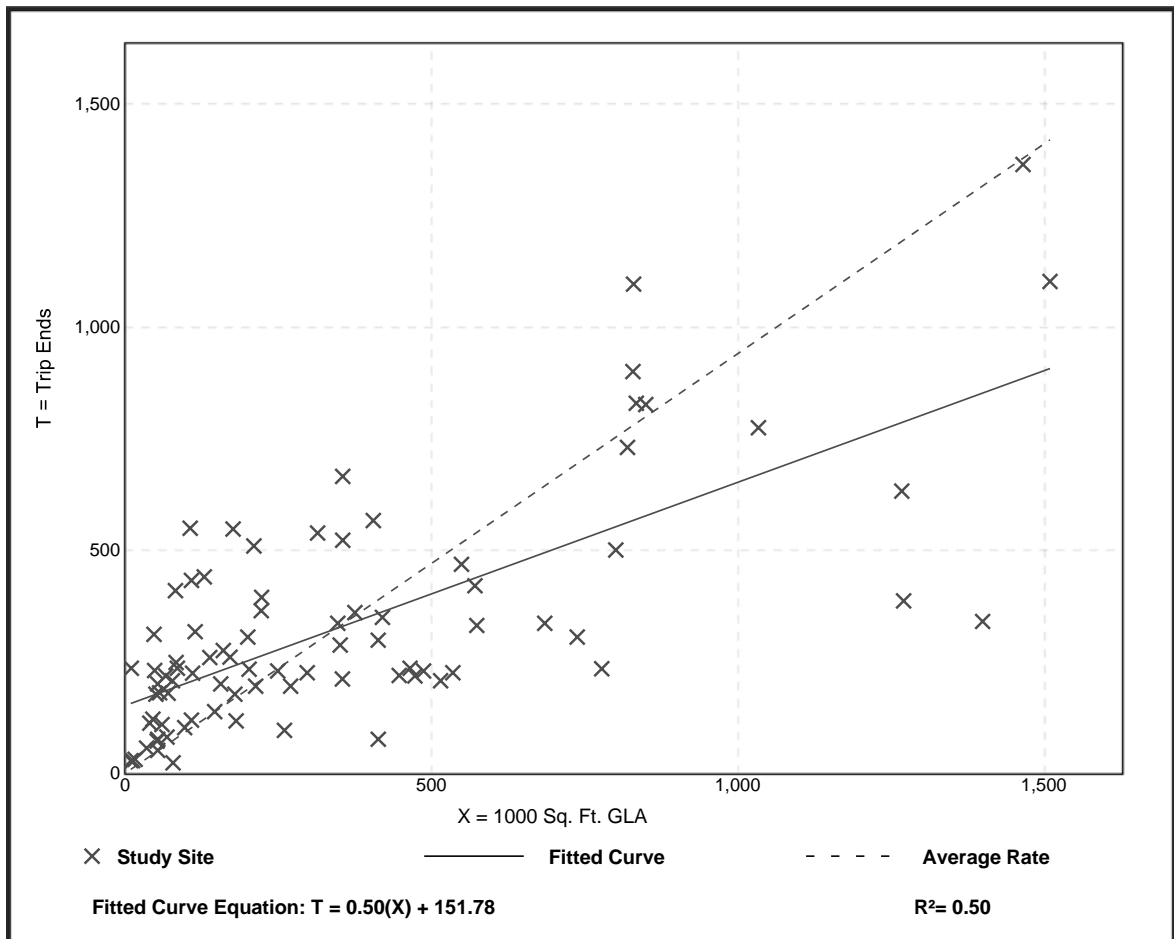
# Shopping Center (820)

**Vehicle Trip Ends vs: 1000 Sq. Ft. GLA**  
**On a: Weekday,**  
**Peak Hour of Adjacent Street Traffic,**  
**One Hour Between 7 and 9 a.m.**  
**Setting/Location: General Urban/Suburban**  
 Number of Studies: 84  
 Avg. 1000 Sq. Ft. GLA: 351  
 Directional Distribution: 62% entering, 38% exiting

## Vehicle Trip Generation per 1000 Sq. Ft. GLA

Average Rate	Range of Rates	Standard Deviation
0.94	0.18 - 23.74	0.87

## Data Plot and Equation



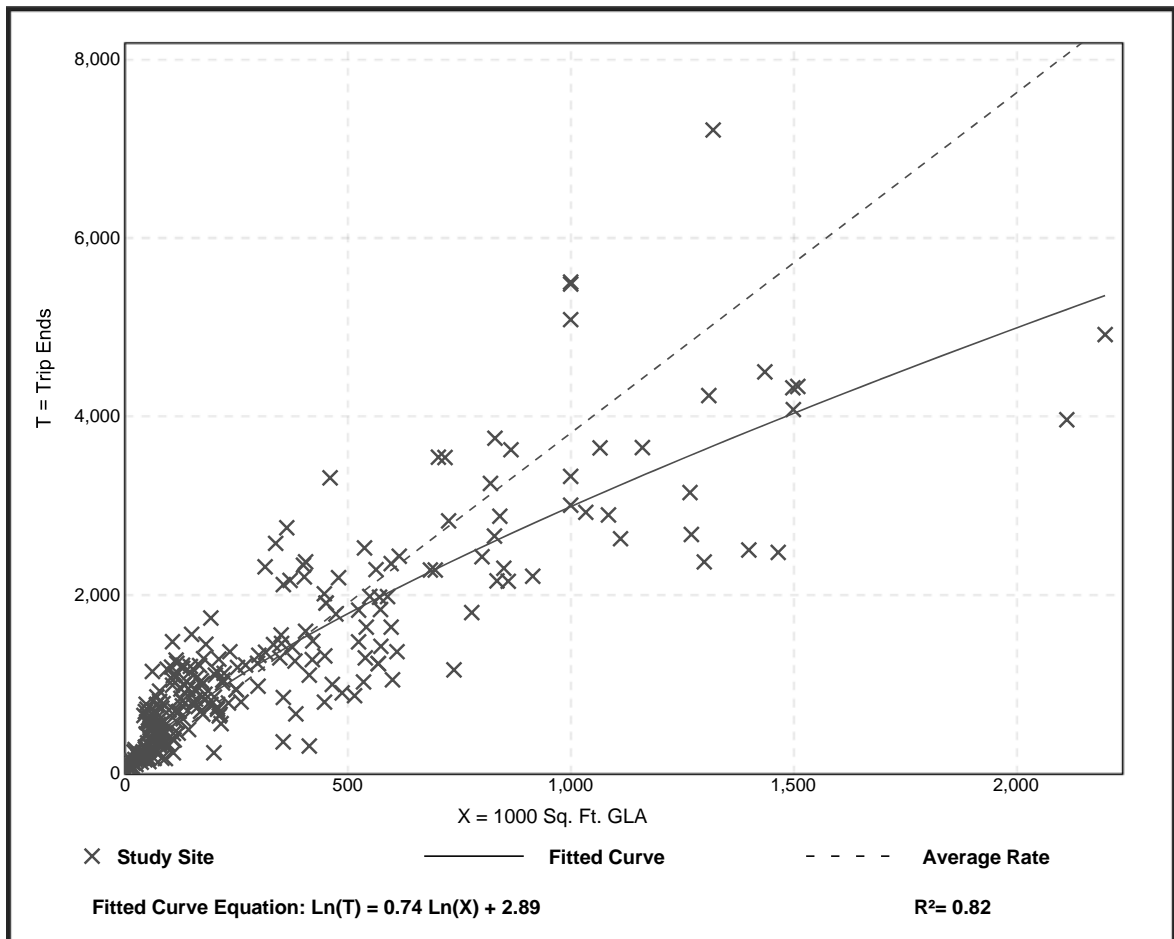
# Shopping Center (820)

**Vehicle Trip Ends vs: 1000 Sq. Ft. GLA**  
**On a: Weekday,**  
**Peak Hour of Adjacent Street Traffic,**  
**One Hour Between 4 and 6 p.m.**  
**Setting/Location: General Urban/Suburban**  
 Number of Studies: 261  
 Avg. 1000 Sq. Ft. GLA: 327  
 Directional Distribution: 48% entering, 52% exiting

## Vehicle Trip Generation per 1000 Sq. Ft. GLA

Average Rate	Range of Rates	Standard Deviation
3.81	0.74 - 18.69	2.04

## Data Plot and Equation





**Proposed Site Development Trip Generation and Distribution**

**Project:** 460 Keil Drive South TIS

**Site:** 460 Keil Drive South, Chatham, Ontario

**Assumed Land Use (1):** Multifamily Housing (Mid-Rise) - ITE No. 221

\*\*Note: Data not filtered based on region.

**Average Vehicle Trip Ends vs.:** Dwelling Units

**ITE Trip Generation Data collected on a:** Weekday

**AM Peak Hour:**  = Average Rate 

26	% Entering
74	% Exiting

**PM Peak Hour:**  = Average Rate 

61	% Entering
39	% Exiting

**Assumed Land Use (2):** Shopping Center - ITE No. 820

\*\*Note: Data not filtered based on region.

**Average Vehicle Trip Ends vs.:** 1000 sq. ft. GLA

**ITE Trip Generation Data collected on a:** Weekday

**AM Peak Hour:**  = Average Rate 

62	% Entering
38	% Exiting

**PM Peak Hour:**  = Average Rate 

48	% Entering
52	% Exiting

<b>Assumed Land Use (1): Multifamily Housing (Mid-Rise) - ITE No. 221</b>				
	<b>No. of Units</b>	<b>Trips Generated</b>	<b>Trips Entering</b>	<b>Trips Exiting</b>
<b>AM Peak</b>	117	42	11	31
<b>PM Peak</b>	117	51	31	20

<b>Assumed Land Use (2): Shopping Center - ITE No. 820</b>				
	<b>1000 sq. ft. GLA</b>	<b>Trips Generated</b>	<b>Trips Entering</b>	<b>Trips Exiting</b>
<b>AM Peak</b>	5.005	5	3	2
<b>PM Peak</b>	5.005	19	9	10

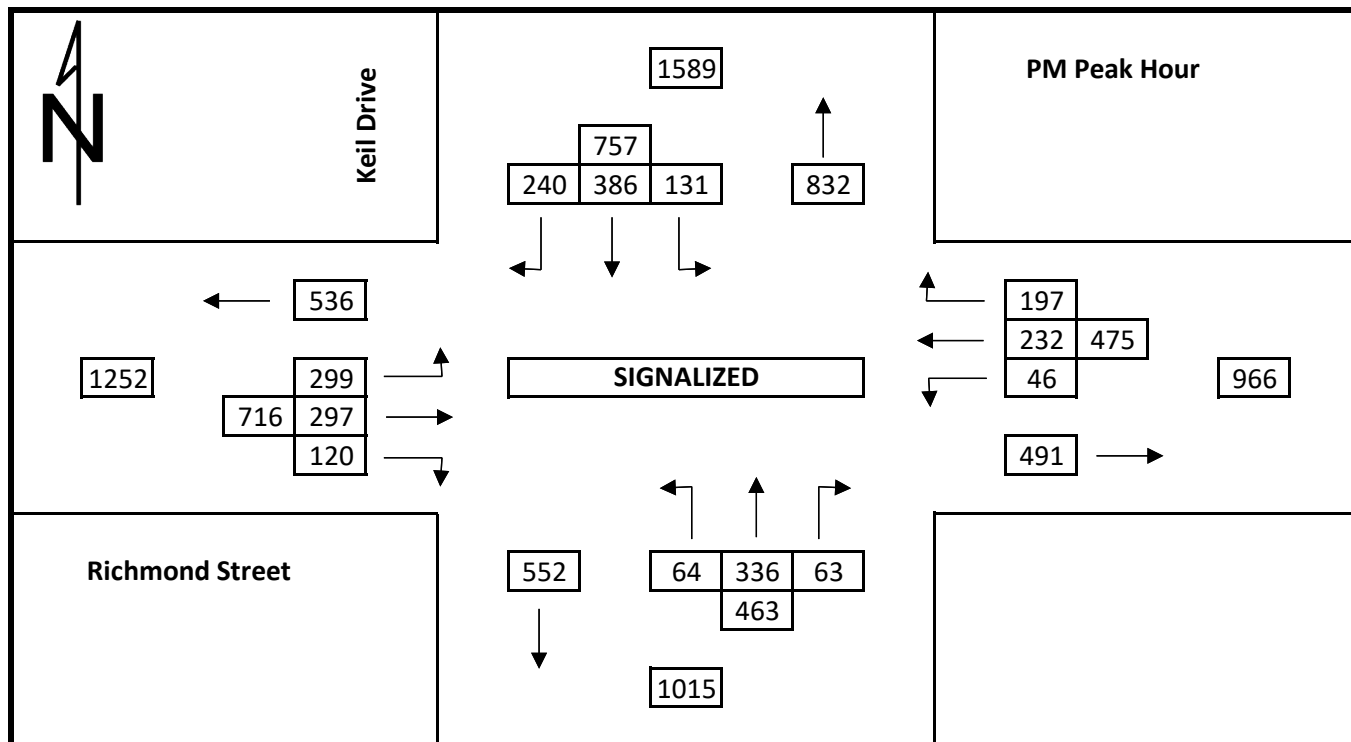
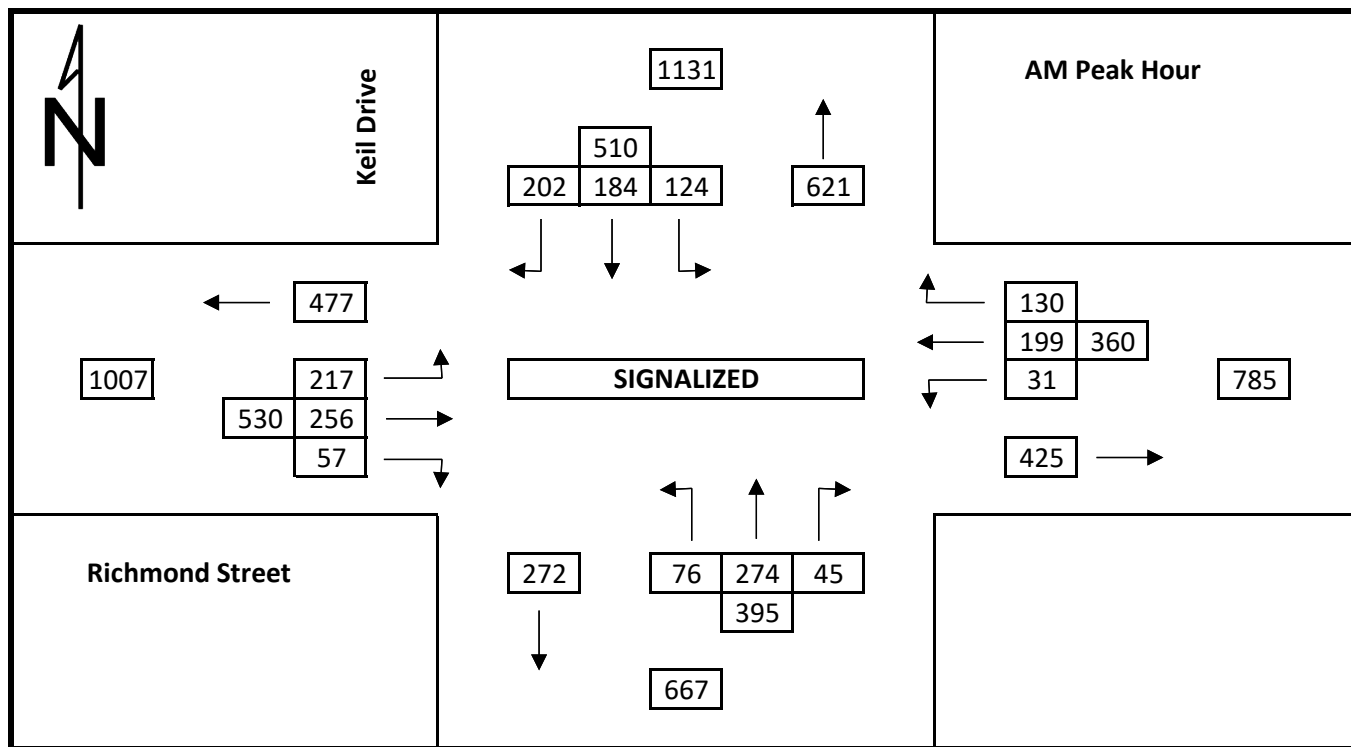
<b>Total Trips</b>		
	<b>Trips Entering</b>	<b>Trips Exiting</b>
<b>AM Peak</b>	14	33
<b>PM Peak</b>	40	31

## **Appendix C**

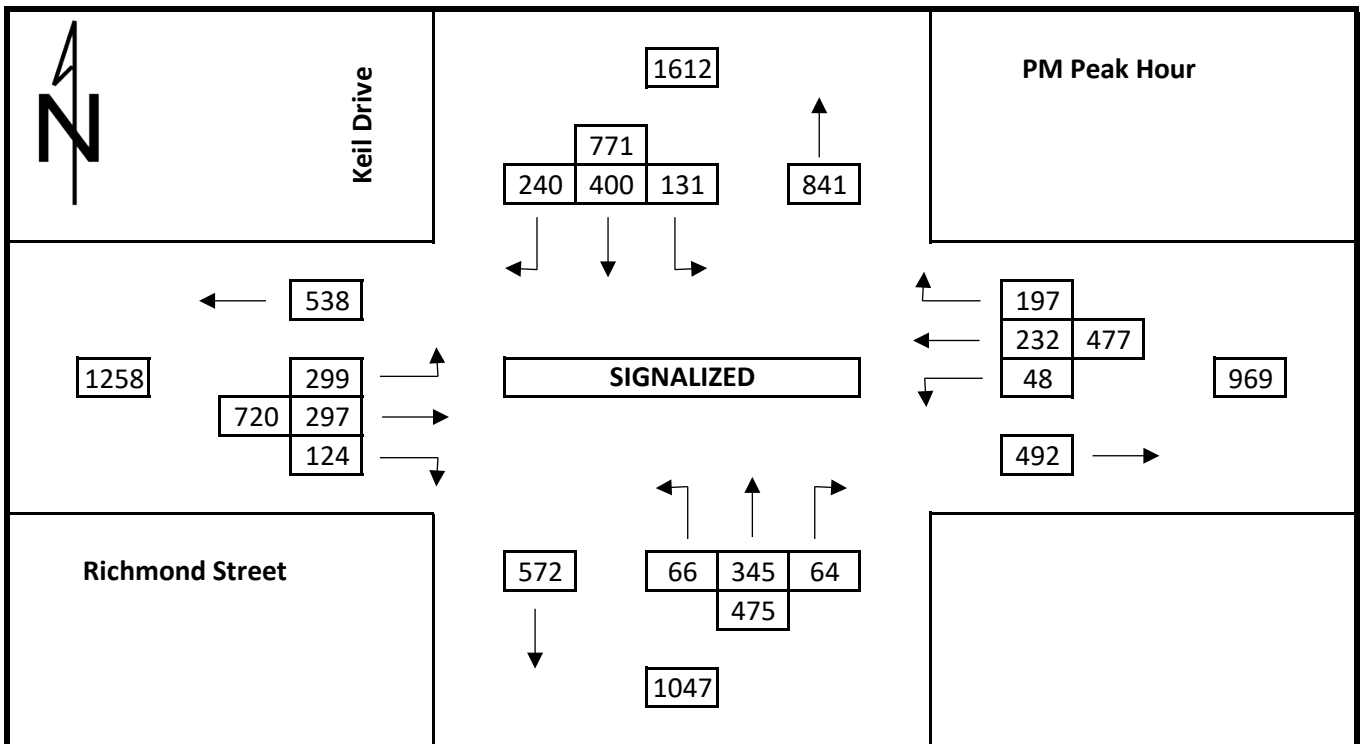
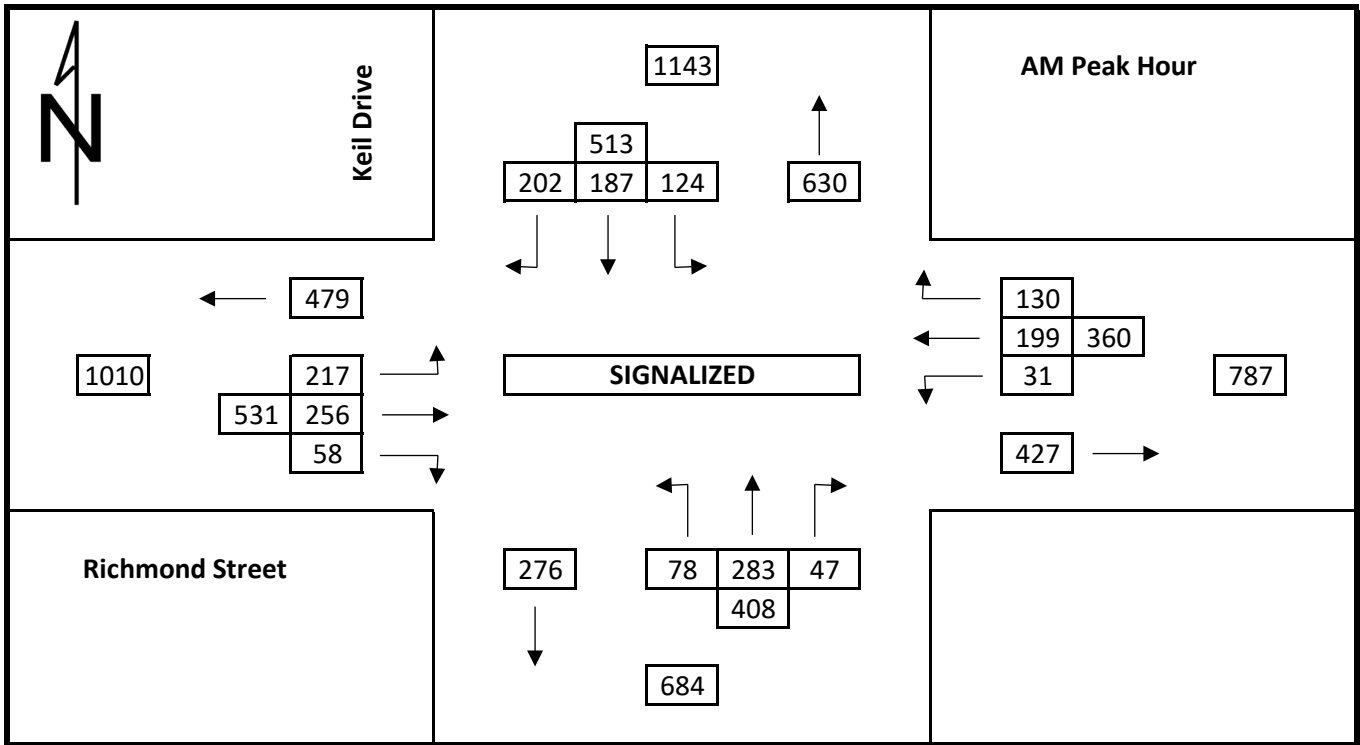
# **TRAFFIC PROJECTION FIGURES**

**Richmond Street at Keil Drive South**  
**Park Avenue West at Keil Drive South**  
**Site Access at Keil Drive South**  
**Site Access at Park Avenue West**

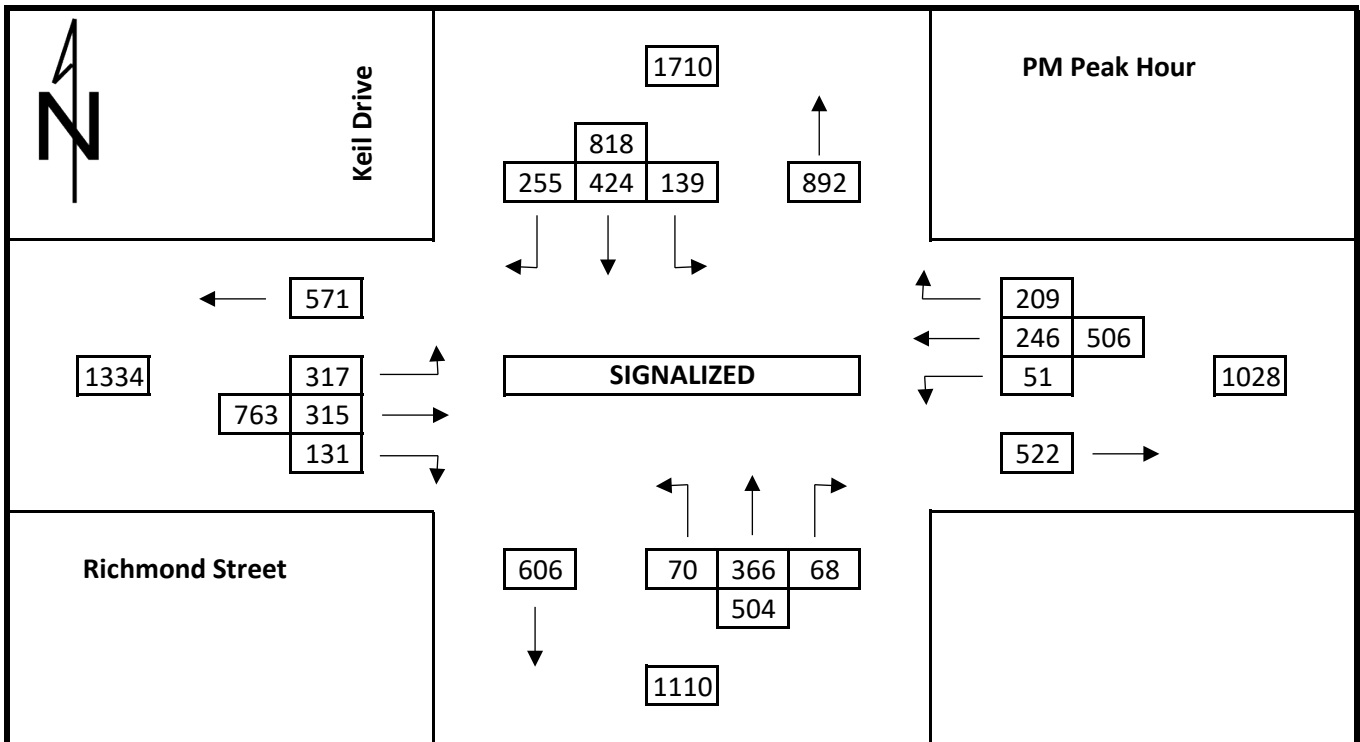
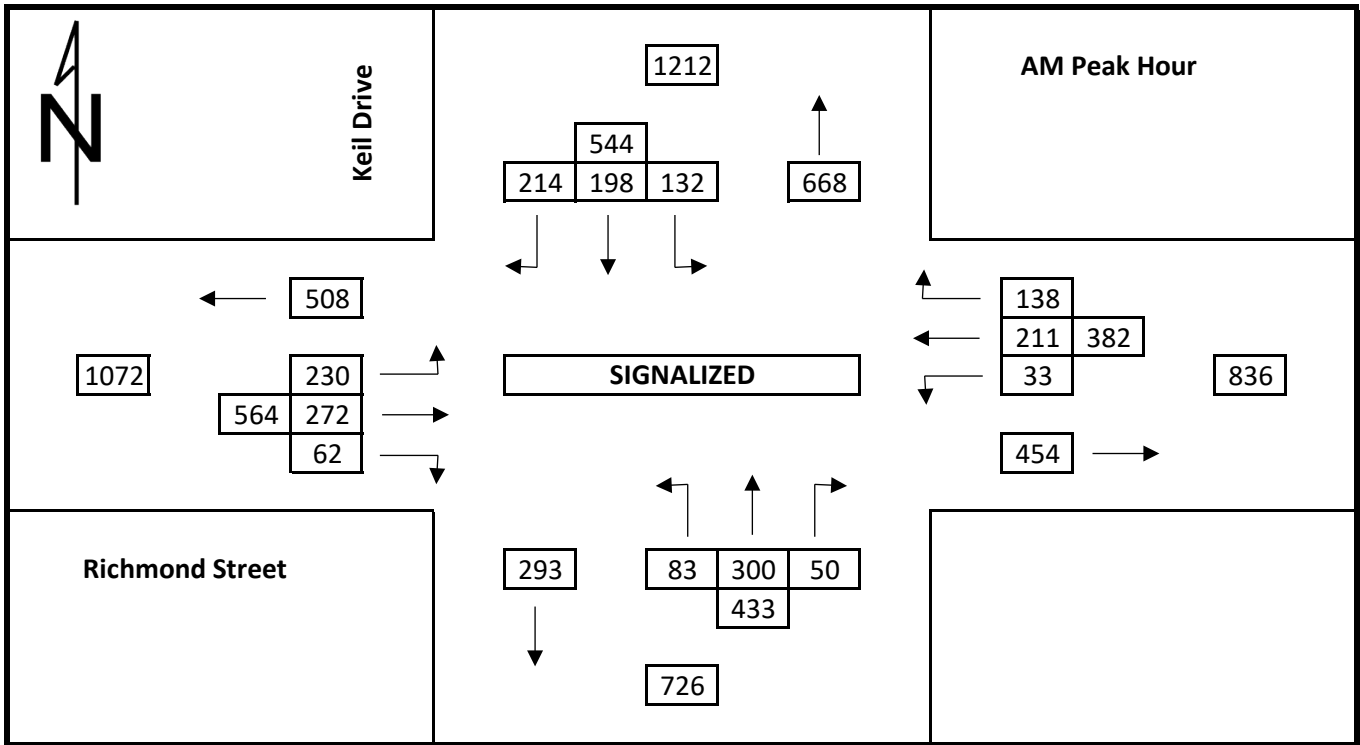
### Existing Traffic Counts Richmond Street at Keil Drive South



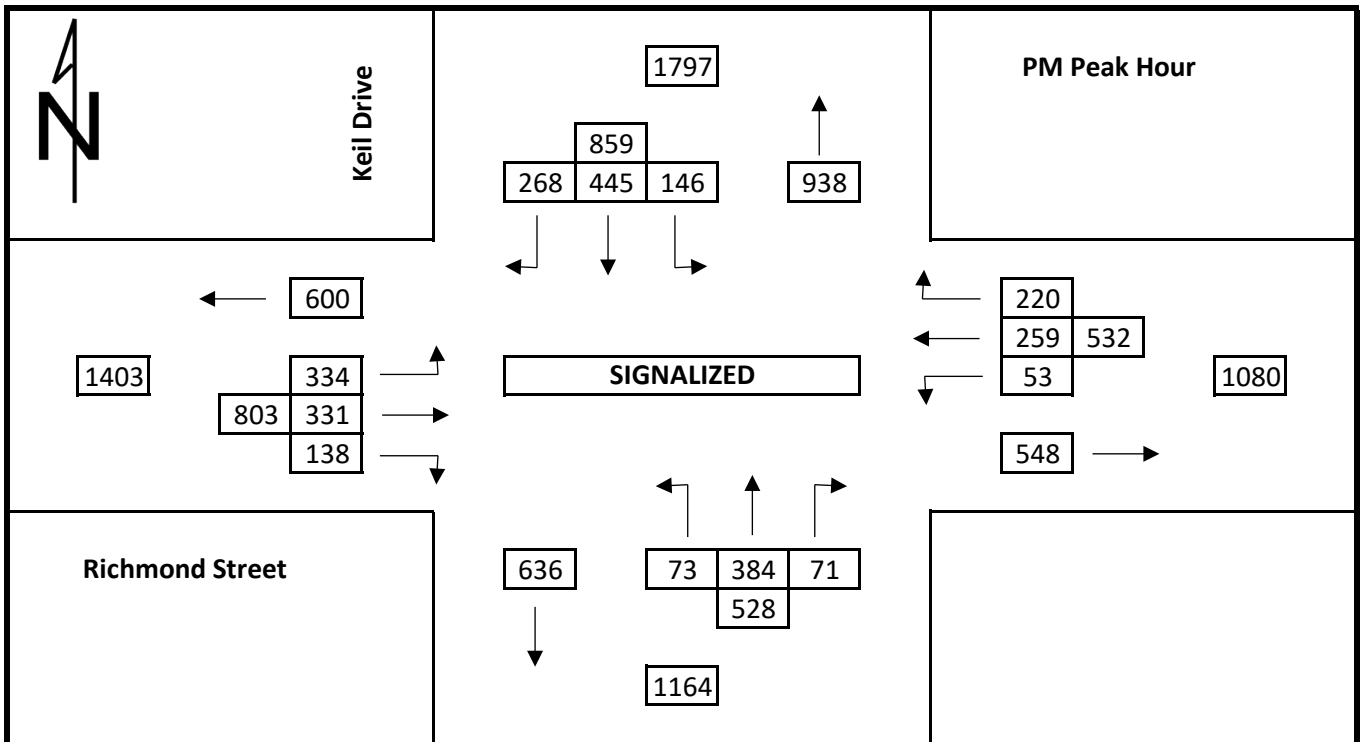
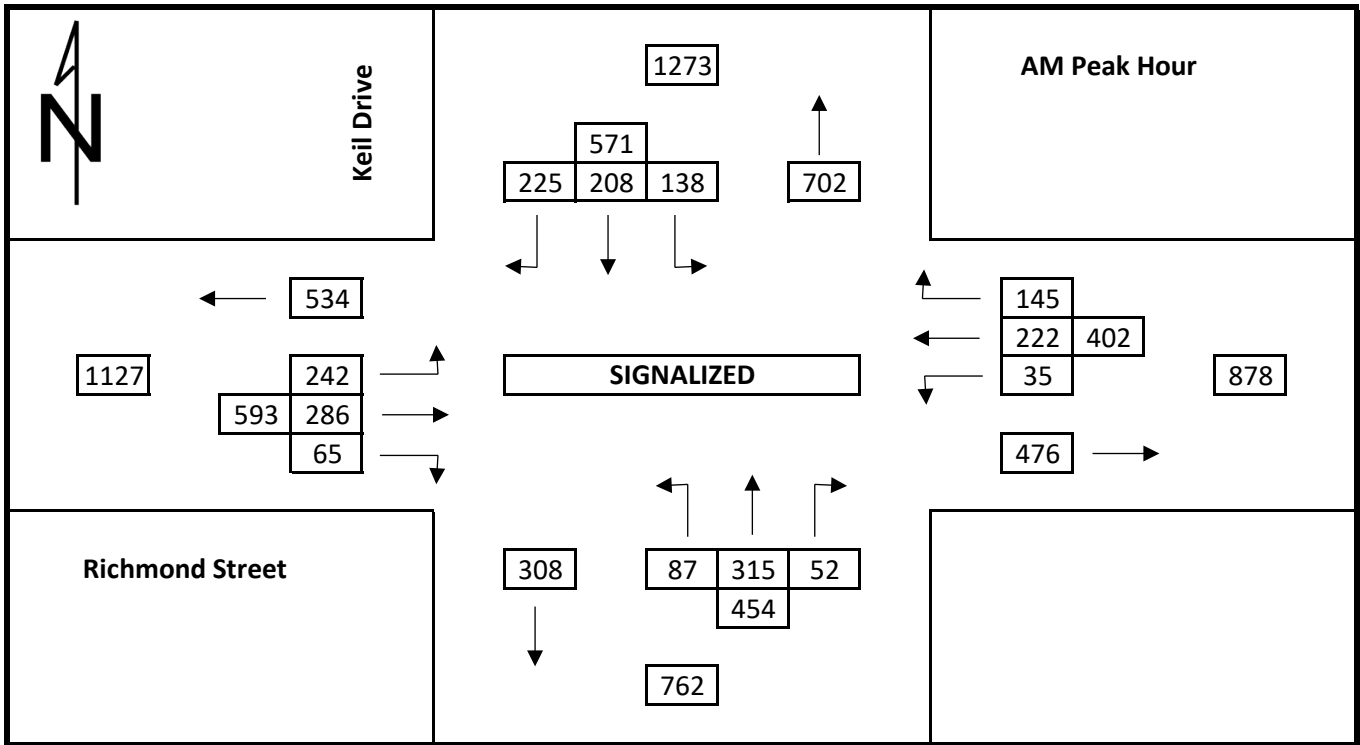
**Existing + Site Generated Traffic**  
 Richmond Street at Keil Drive South



**Total Traffic 2026**  
 Richmond Street at Keil Drive South

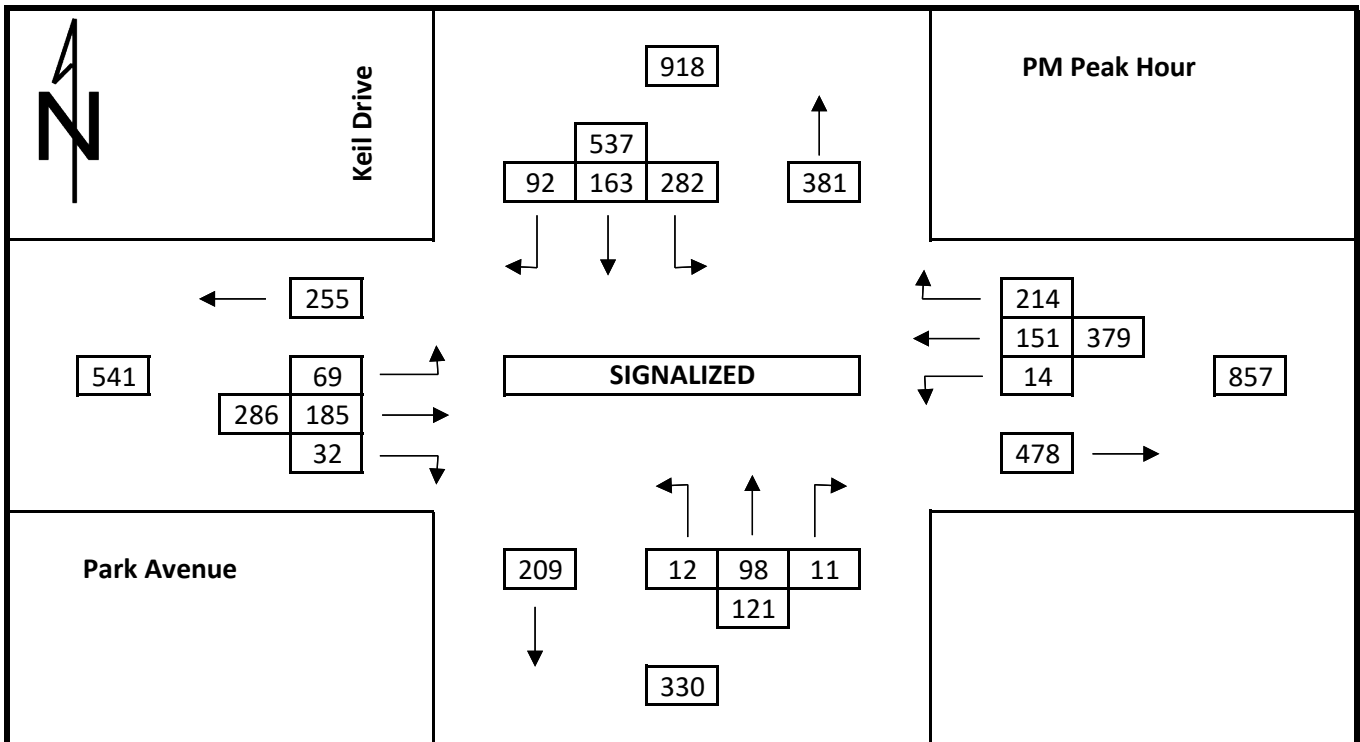
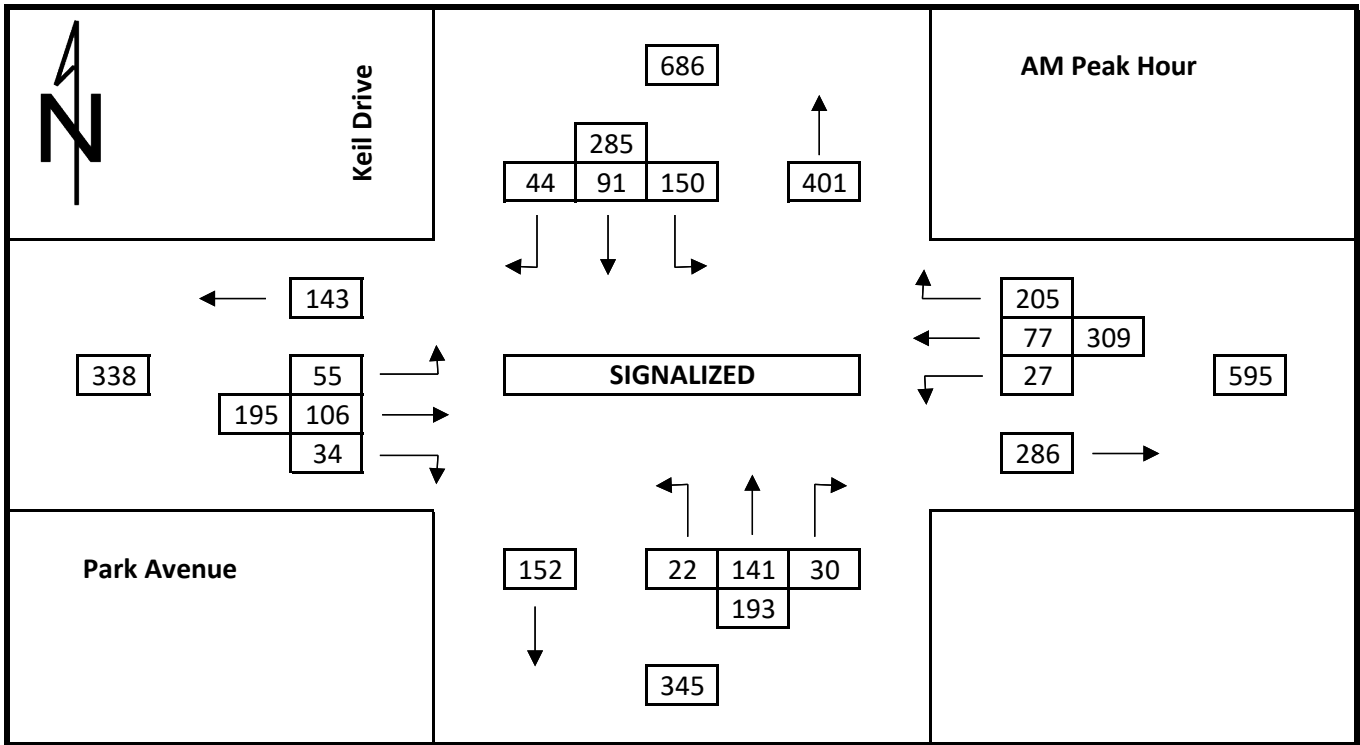


**Total Traffic 2031**  
 Richmond Street at Keil Drive South

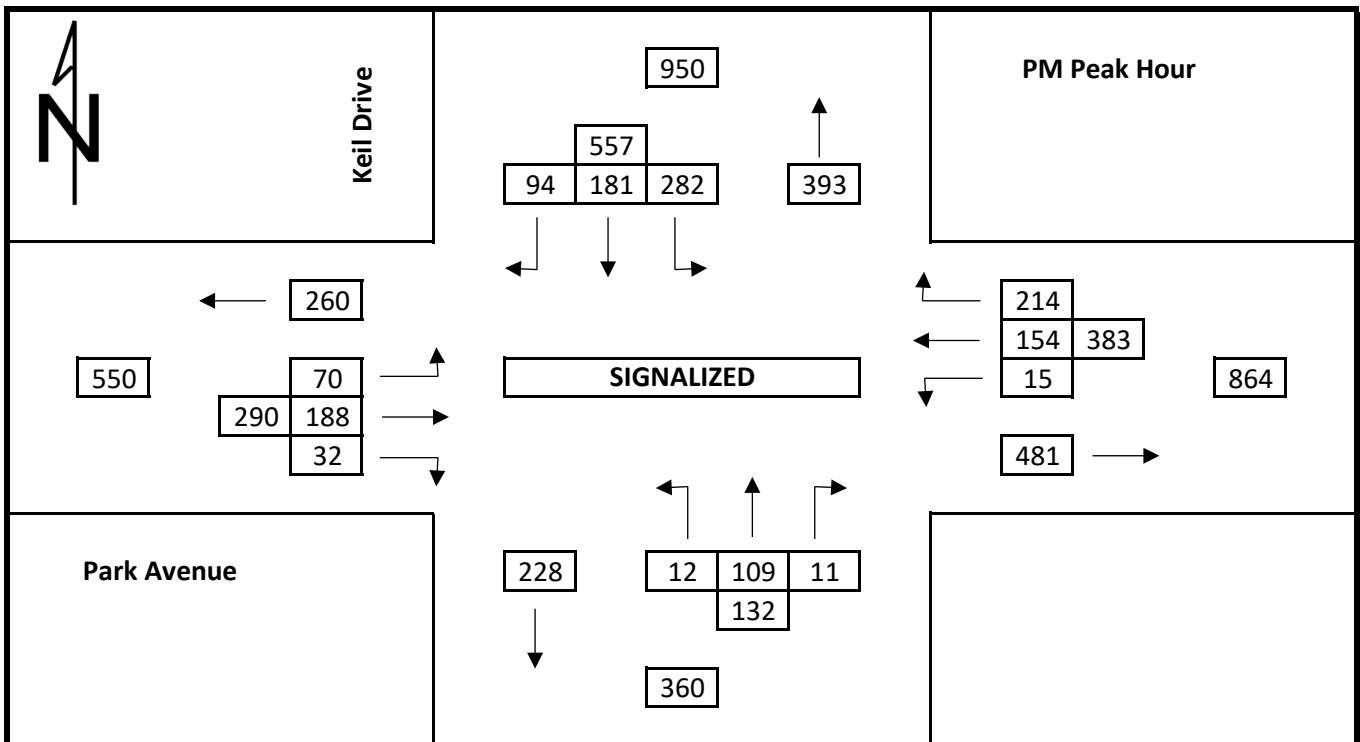
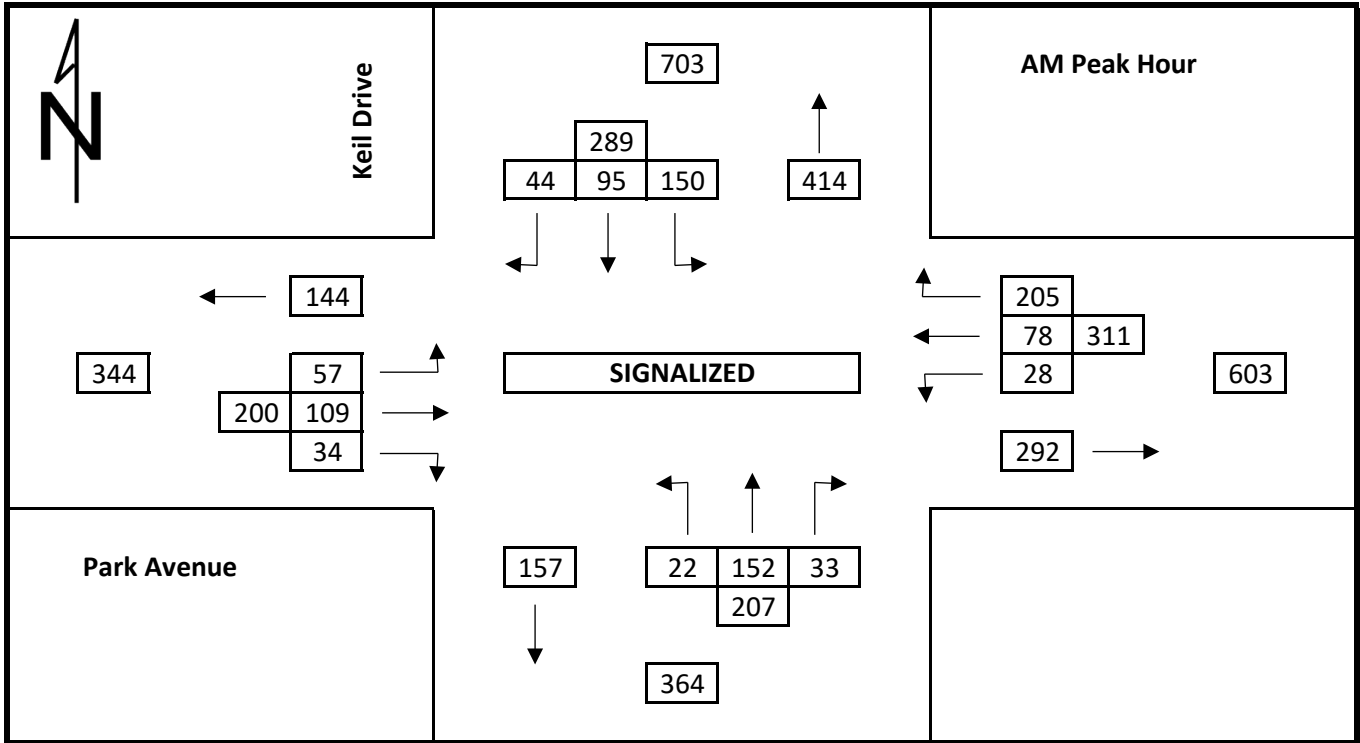


## Existing Traffic Counts

### Park Avenue West at Keil Drive South

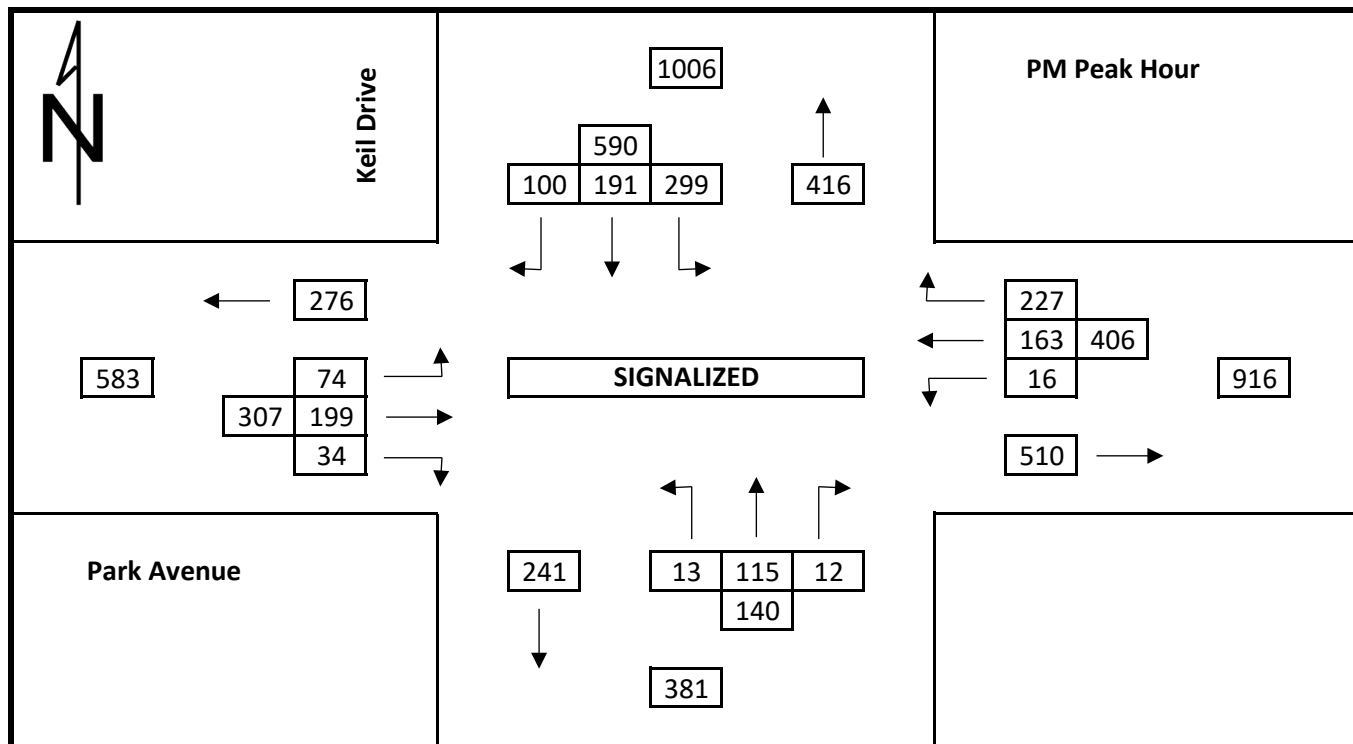
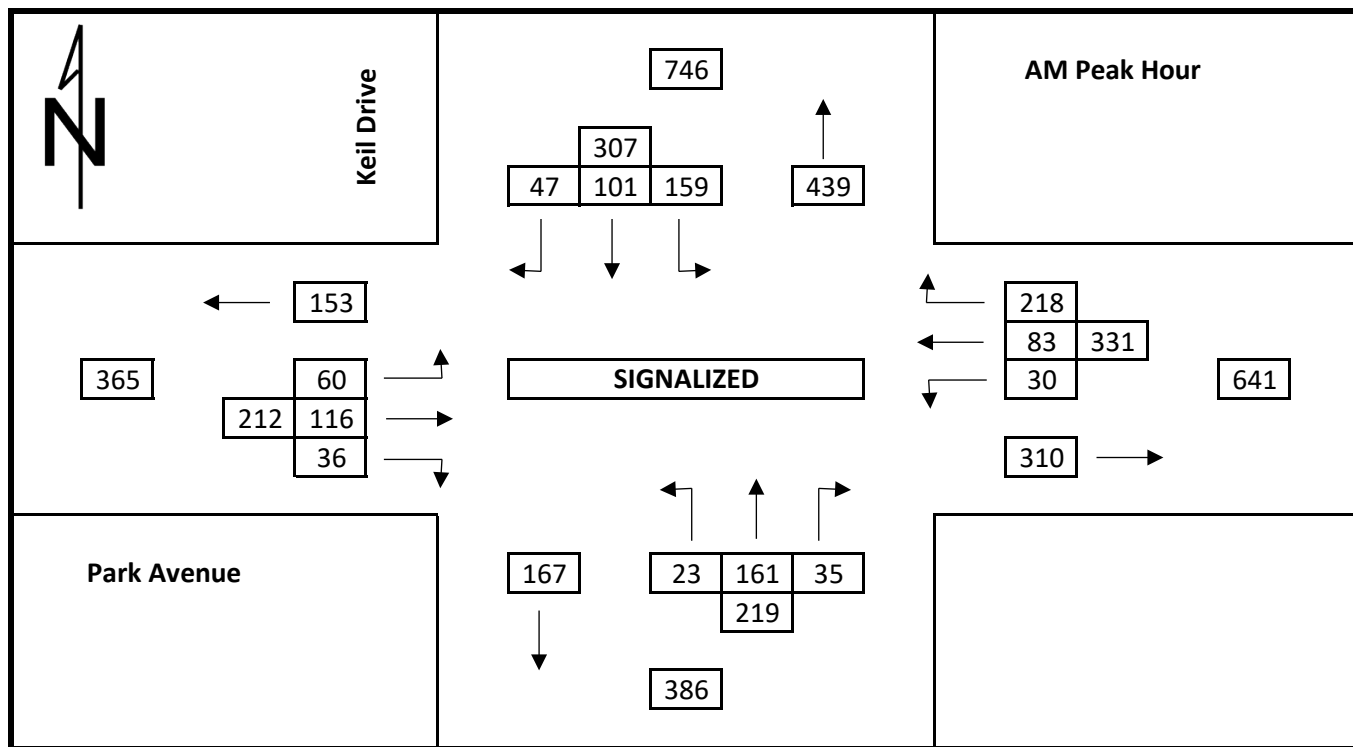


**Existing + Site Generated Traffic**  
 Park Avenue West at Keil Drive South

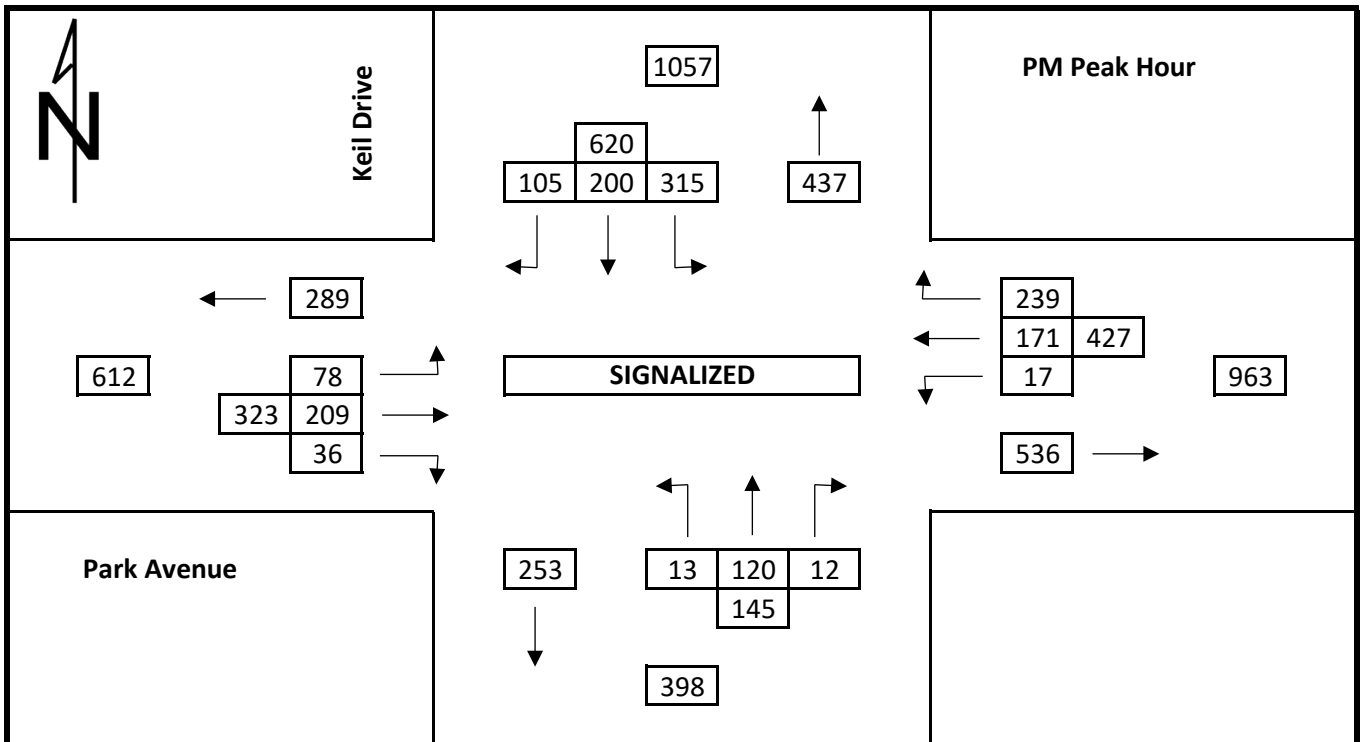
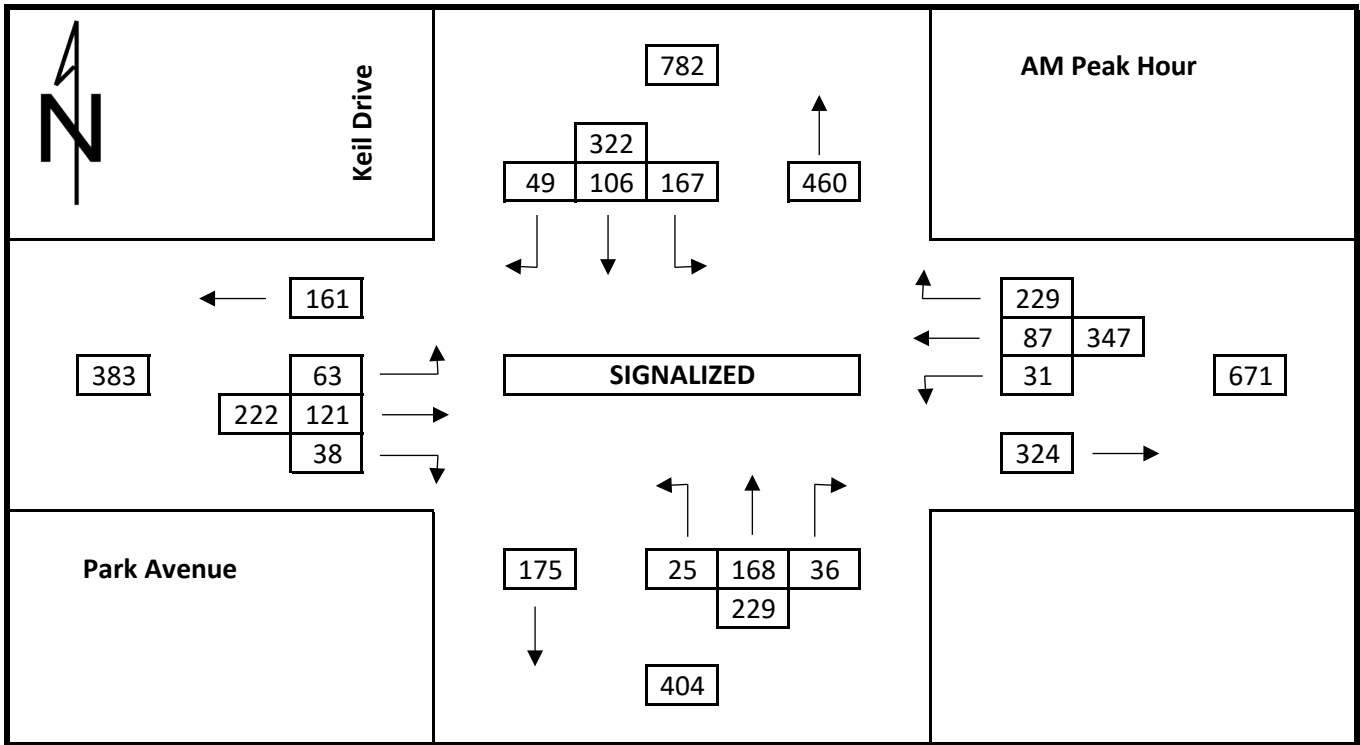




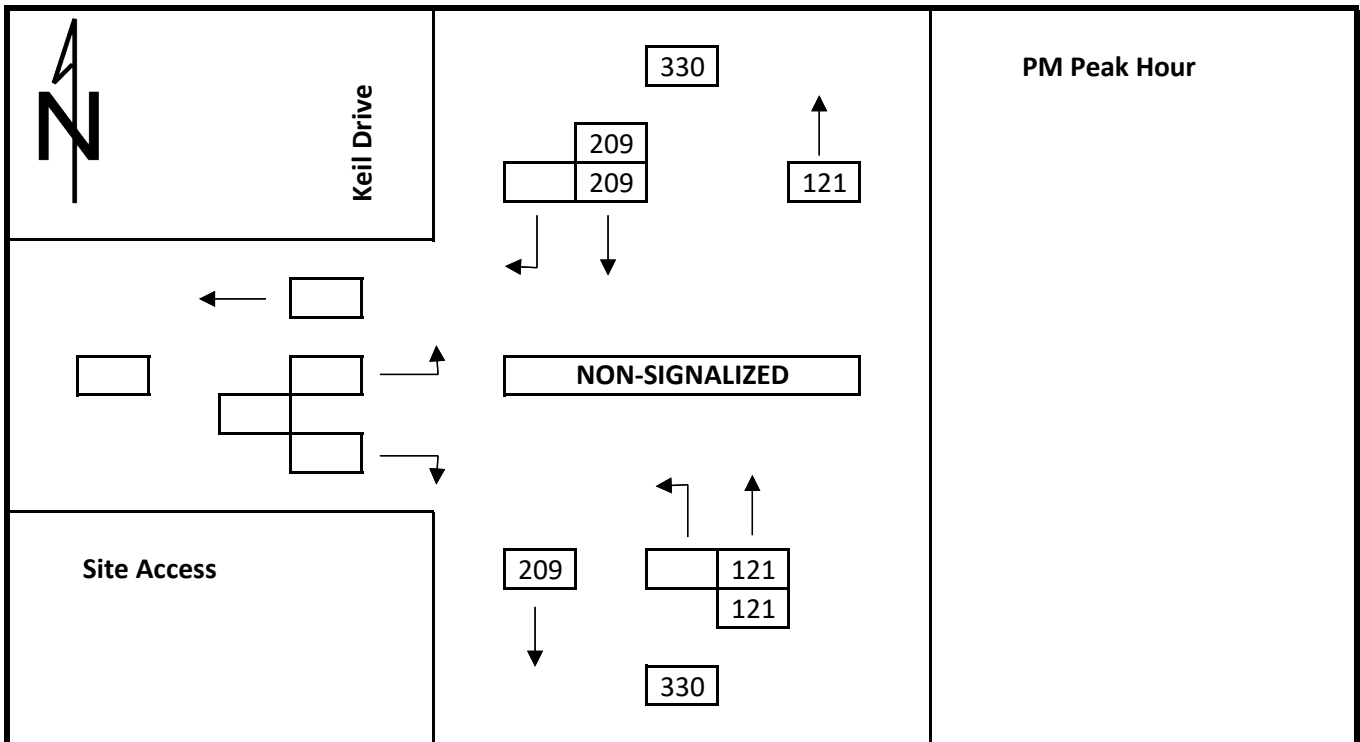
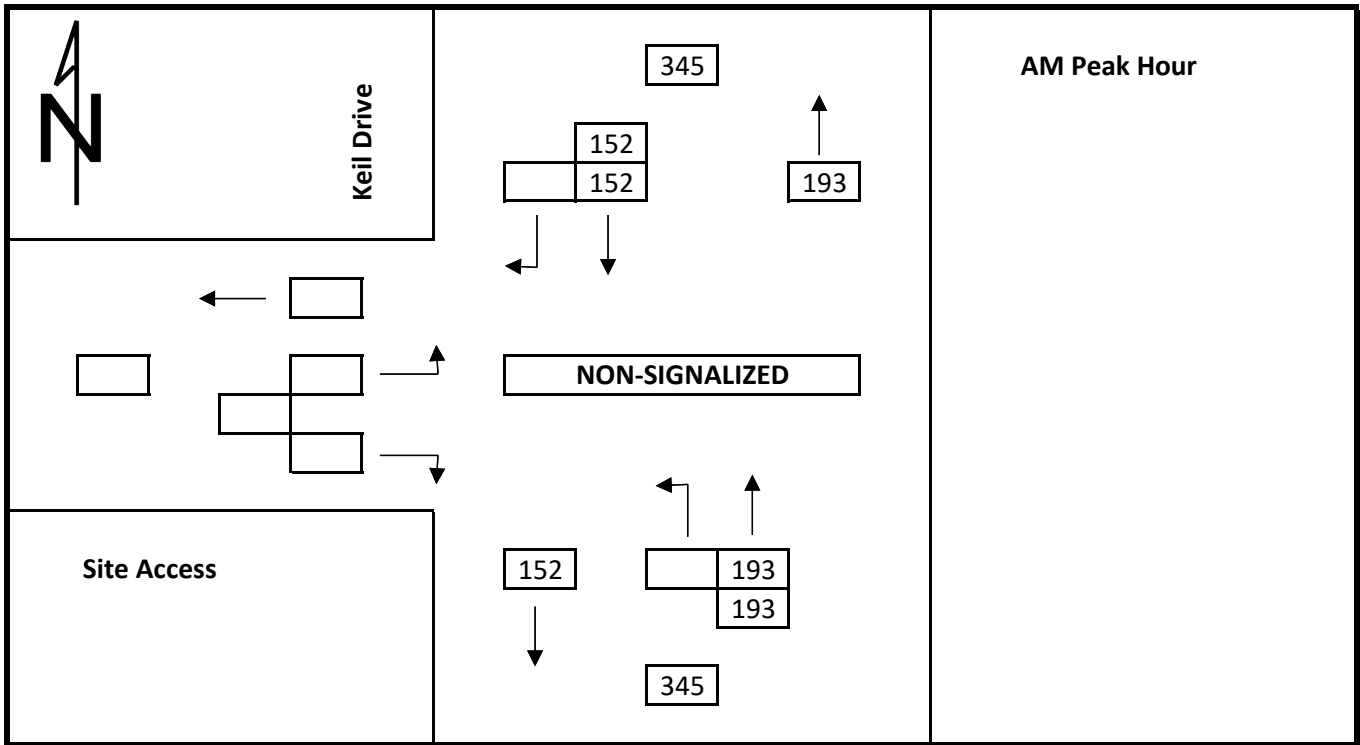
### Total Traffic 2026 Park Avenue West at Keil Drive South



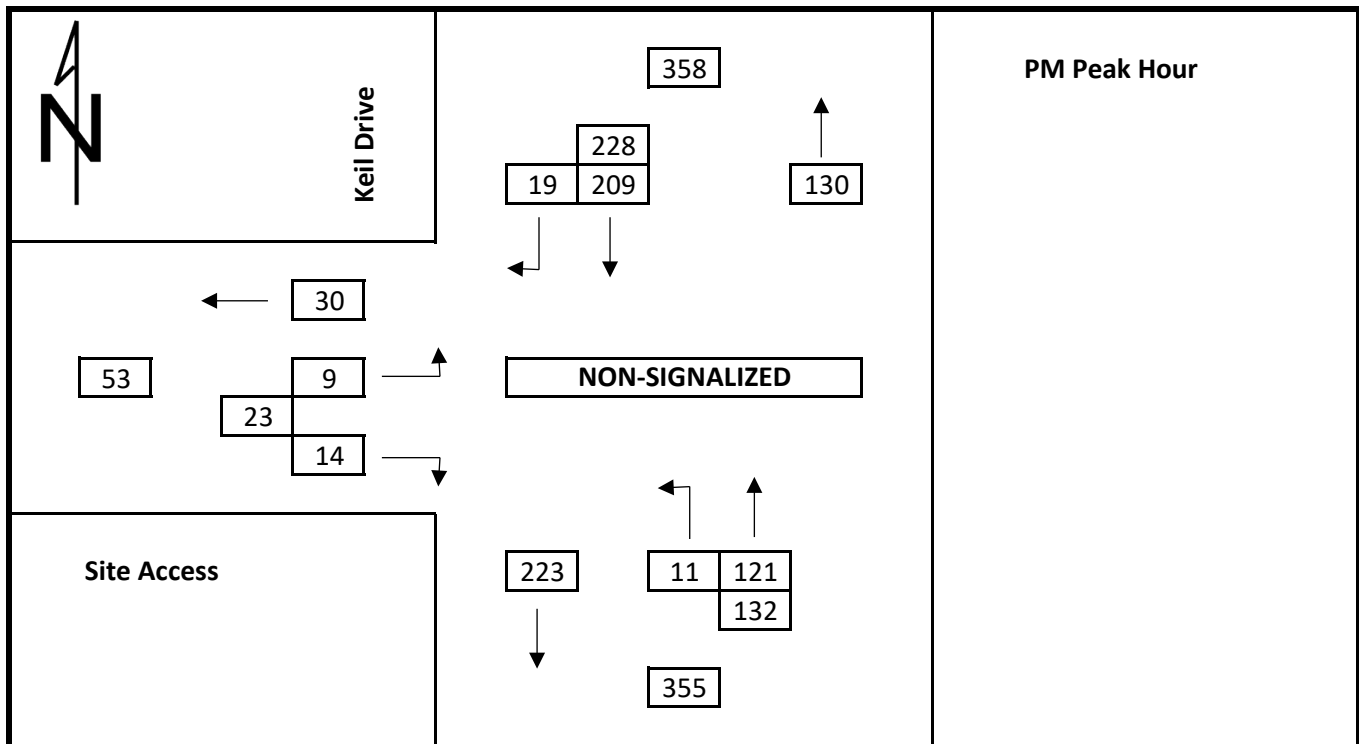
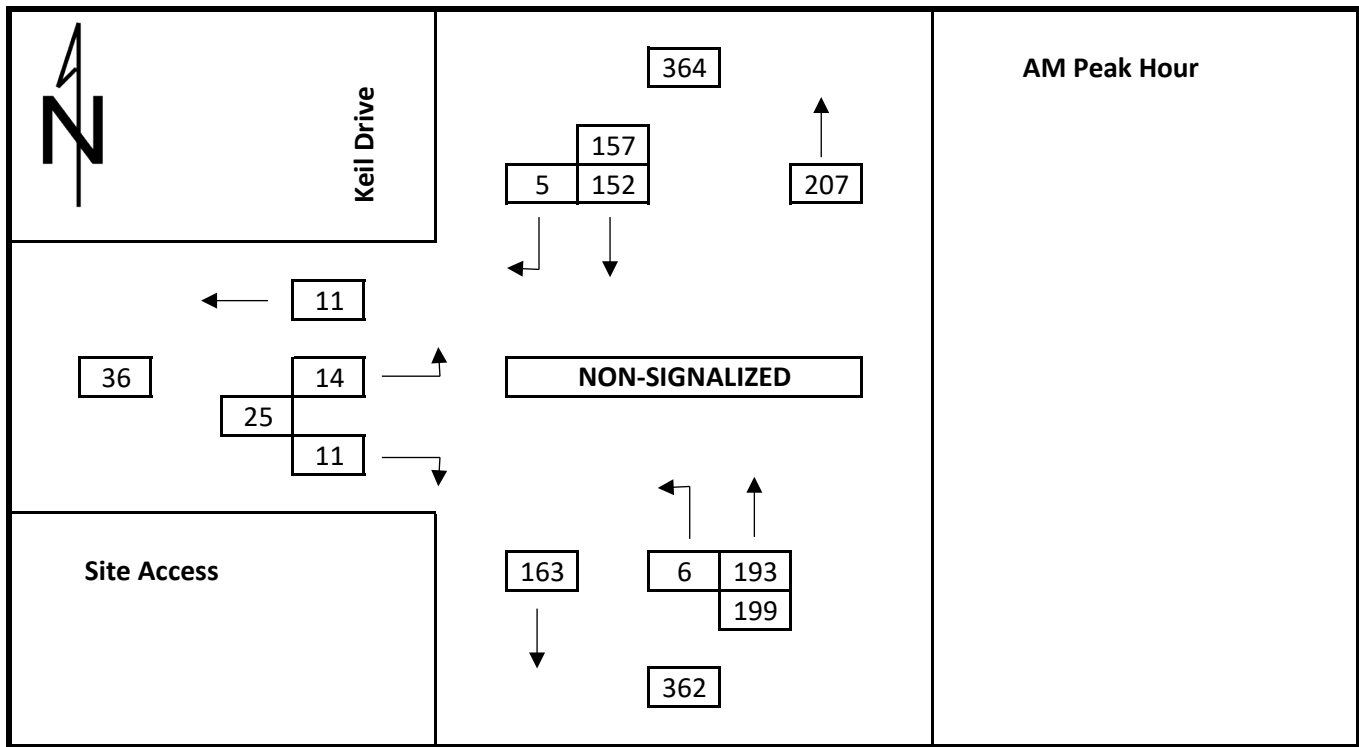
**Total Traffic 2031**  
Park Avenue West at Keil Drive South



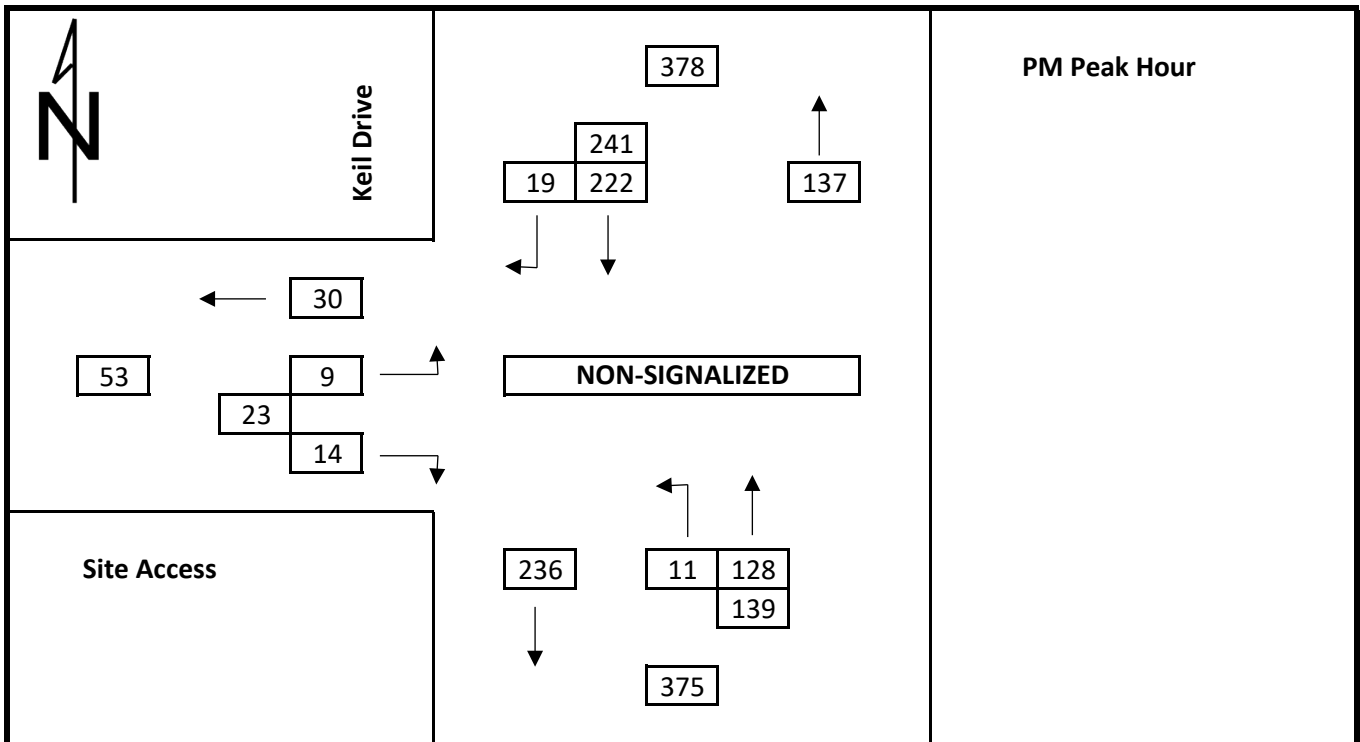
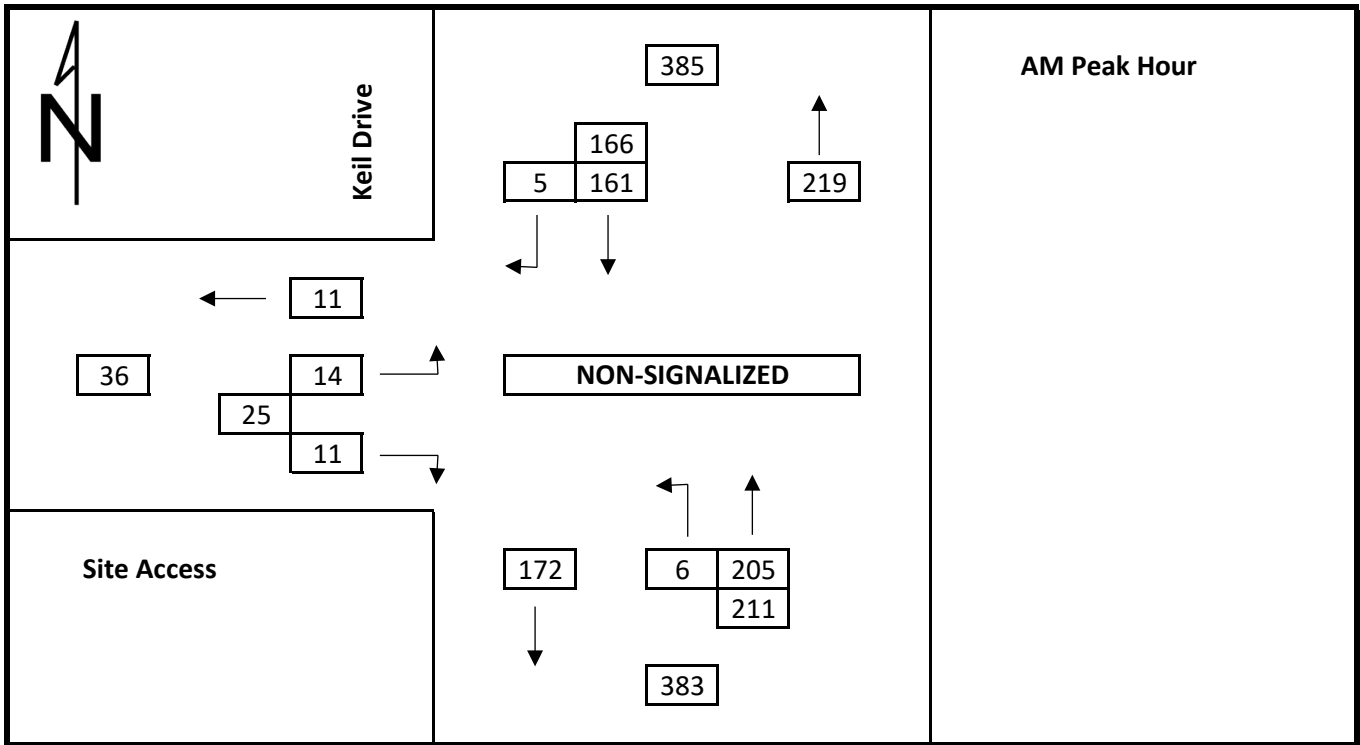
**Existing Traffic Counts**  
 Site Access at Keil Drive South



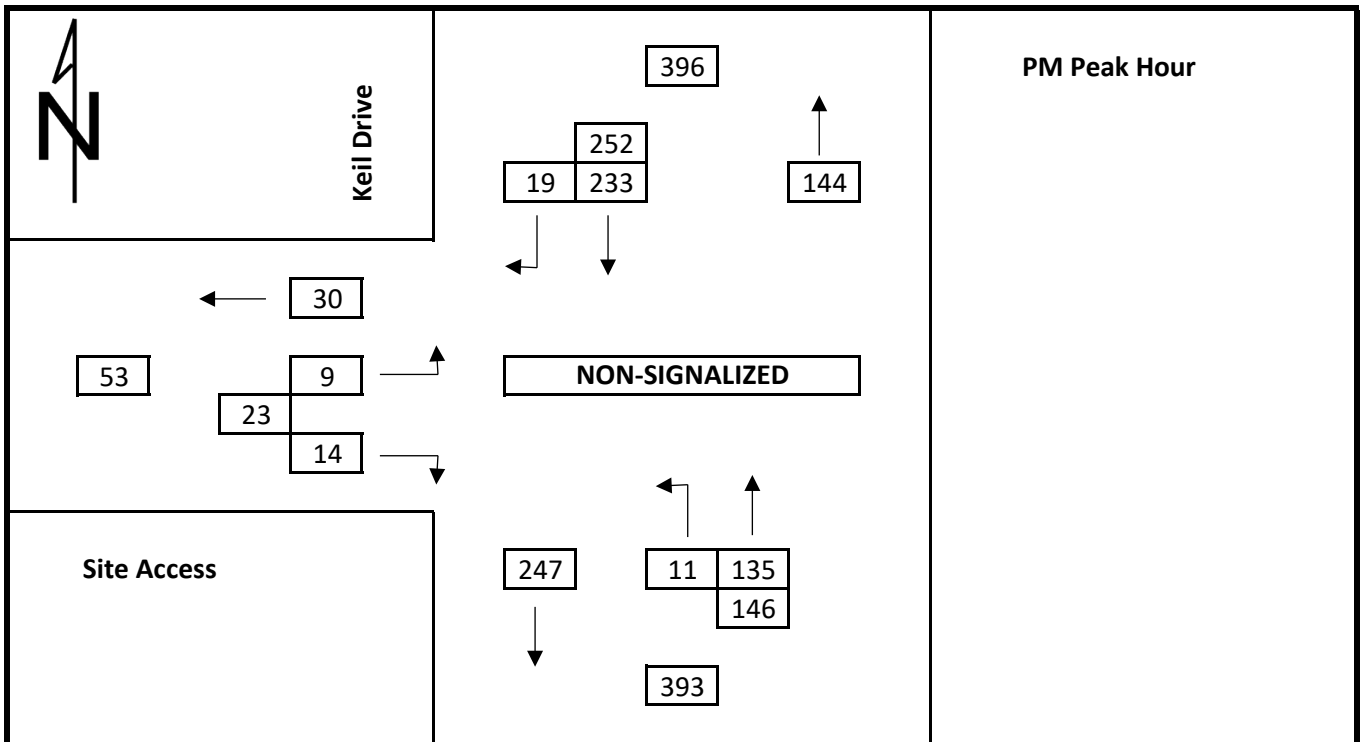
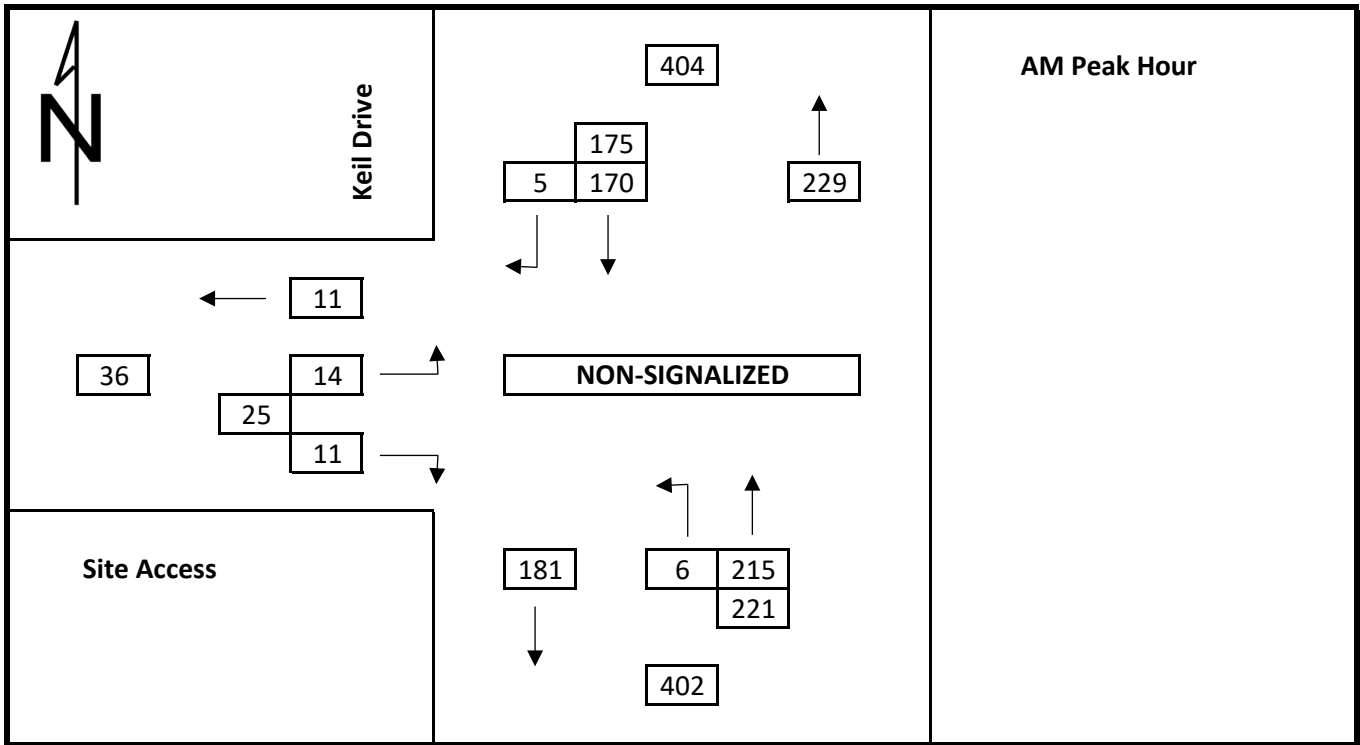
**Existing + Site Generated Traffic**  
 Site Access at Keil Drive South



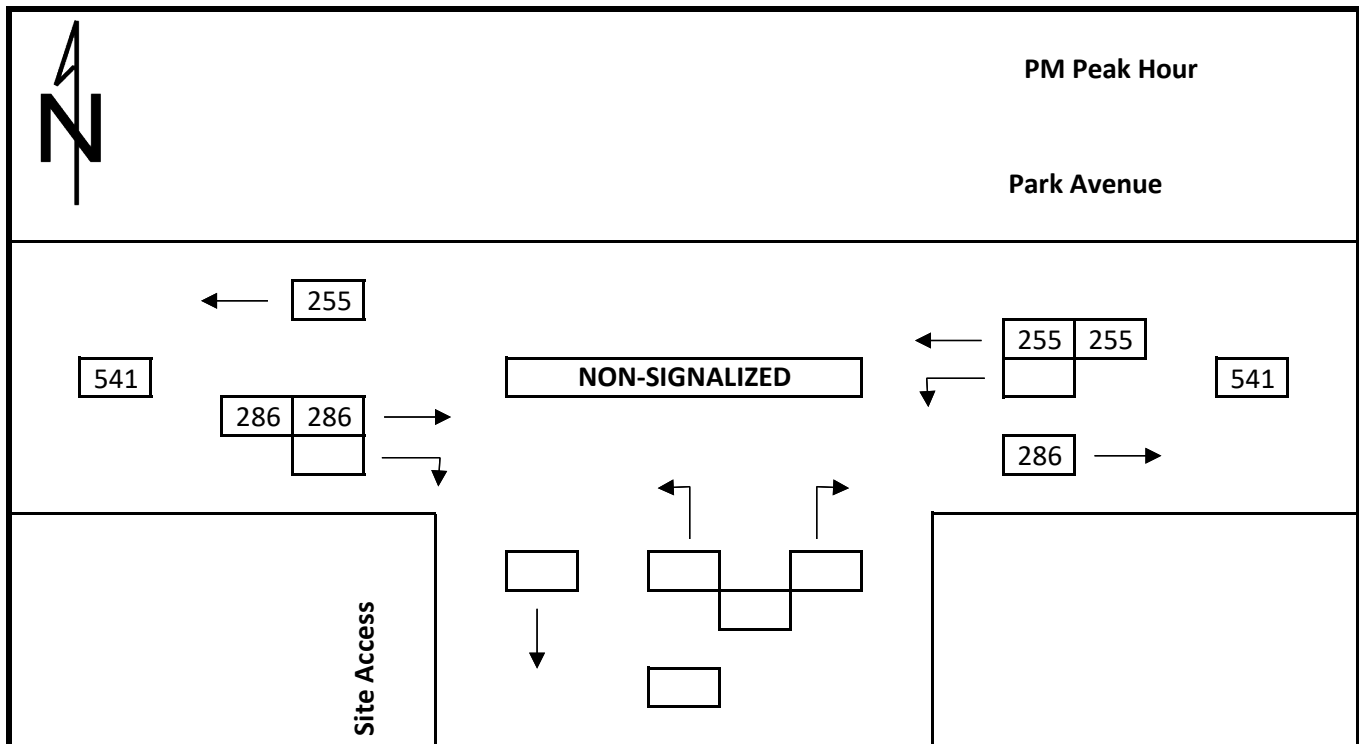
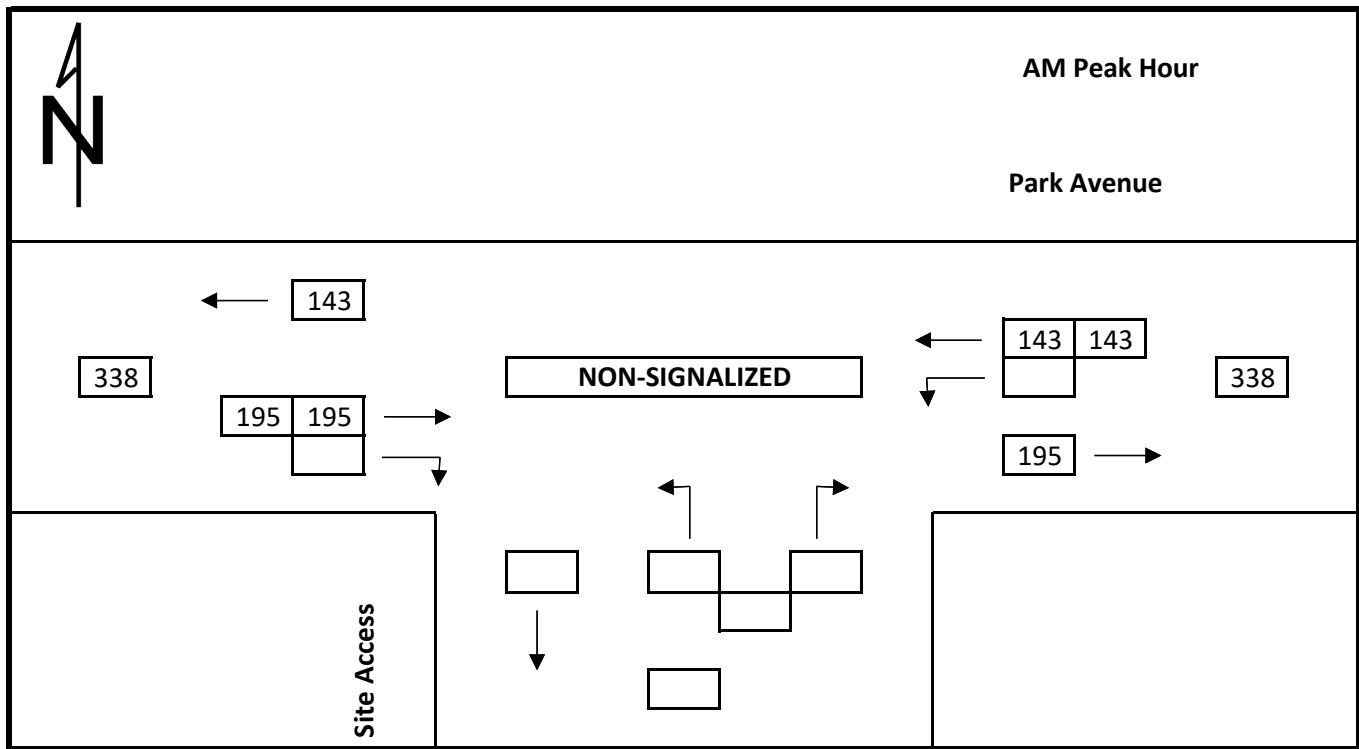
**Total Traffic 2026**  
 Site Access at Keil Drive South



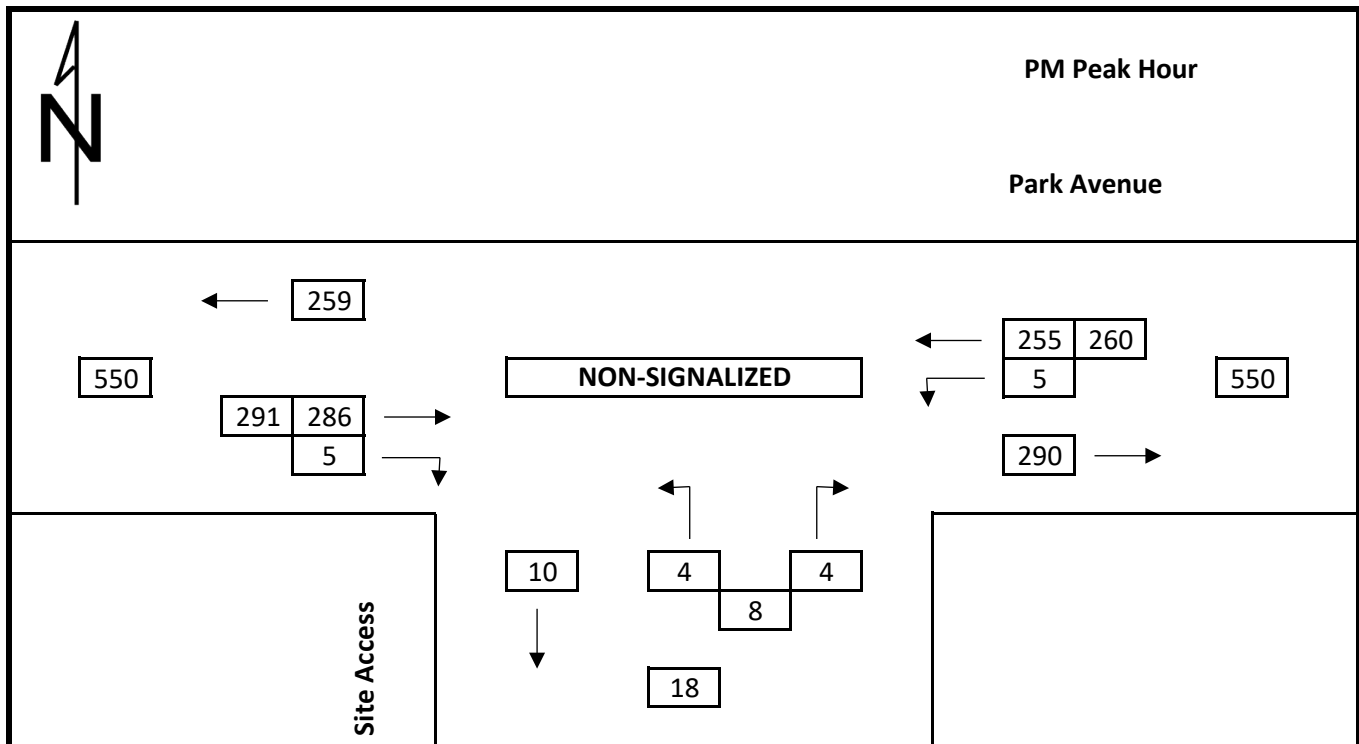
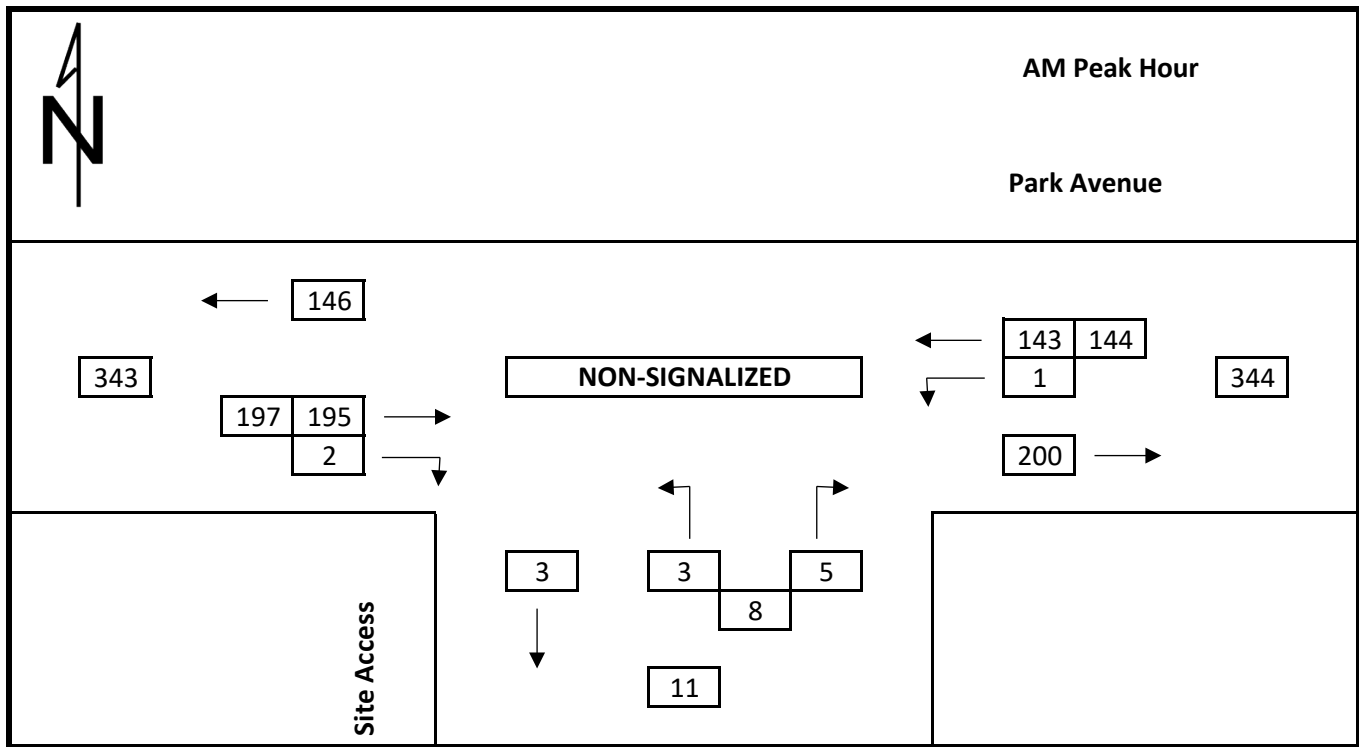
**Total Traffic 2031**  
 Site Access at Keil Drive South



Existing Traffic Counts  
Site Access at Park Avenue West

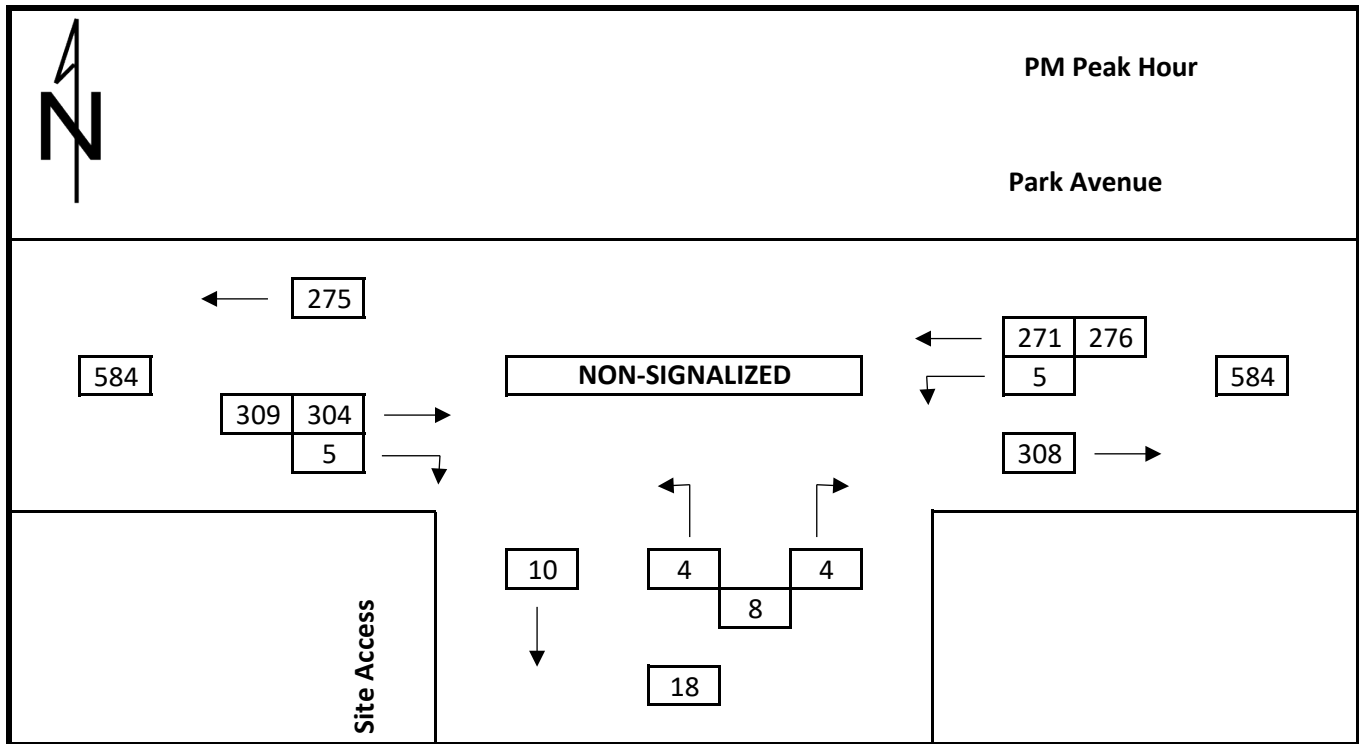
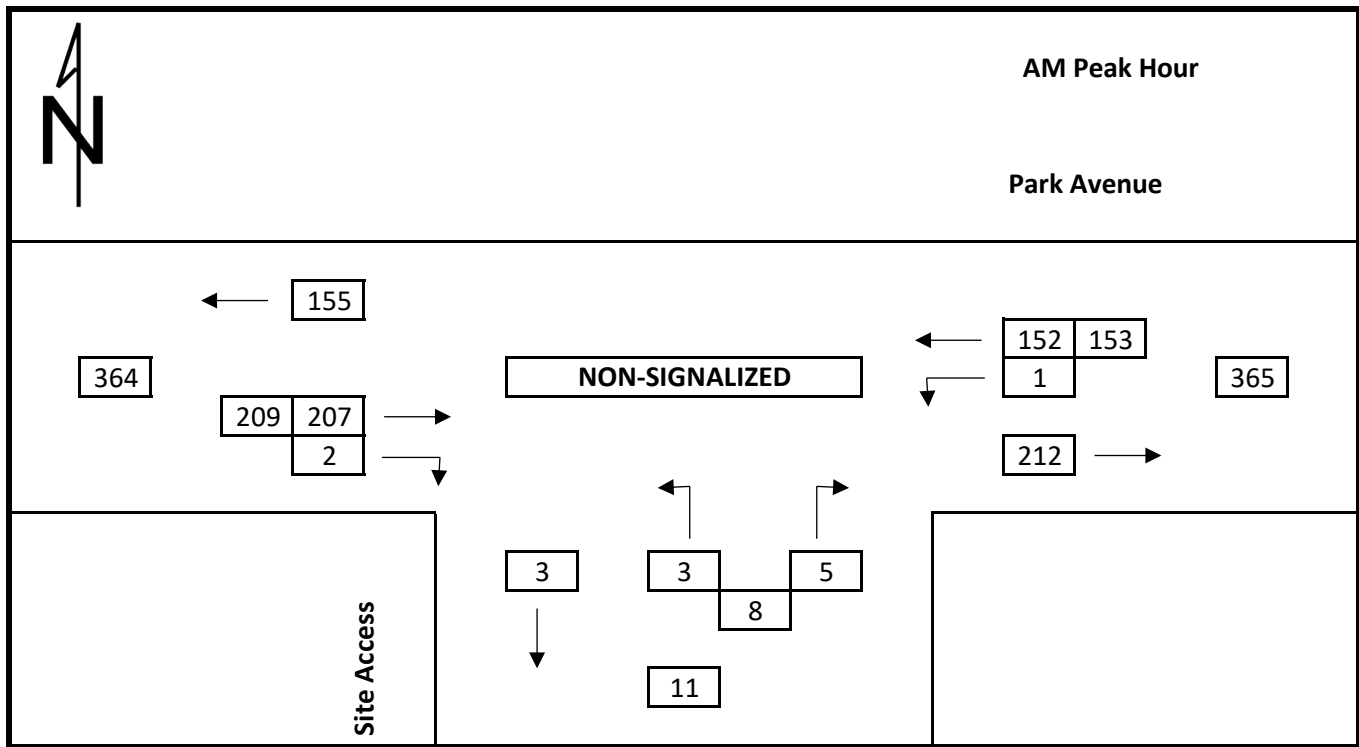


**Existing + Site Generated Traffic**  
 Site Access at Park Avenue West

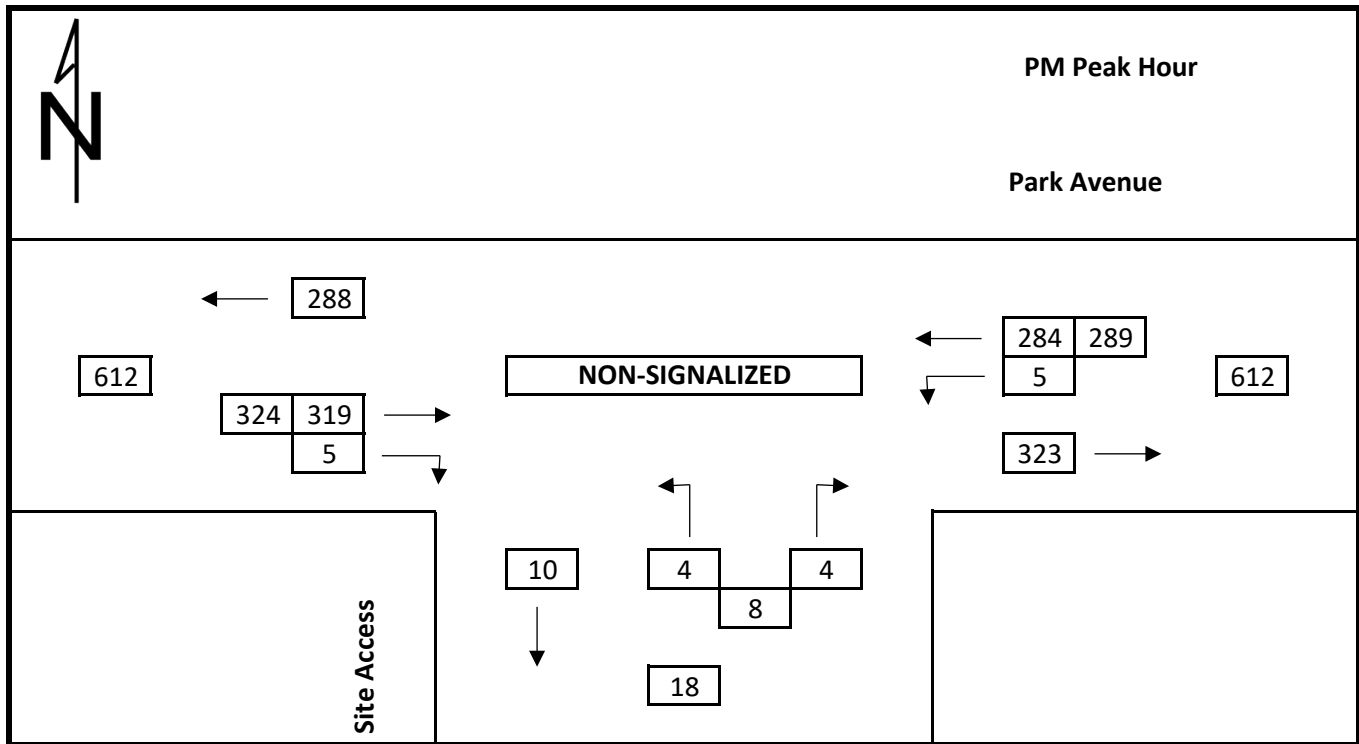
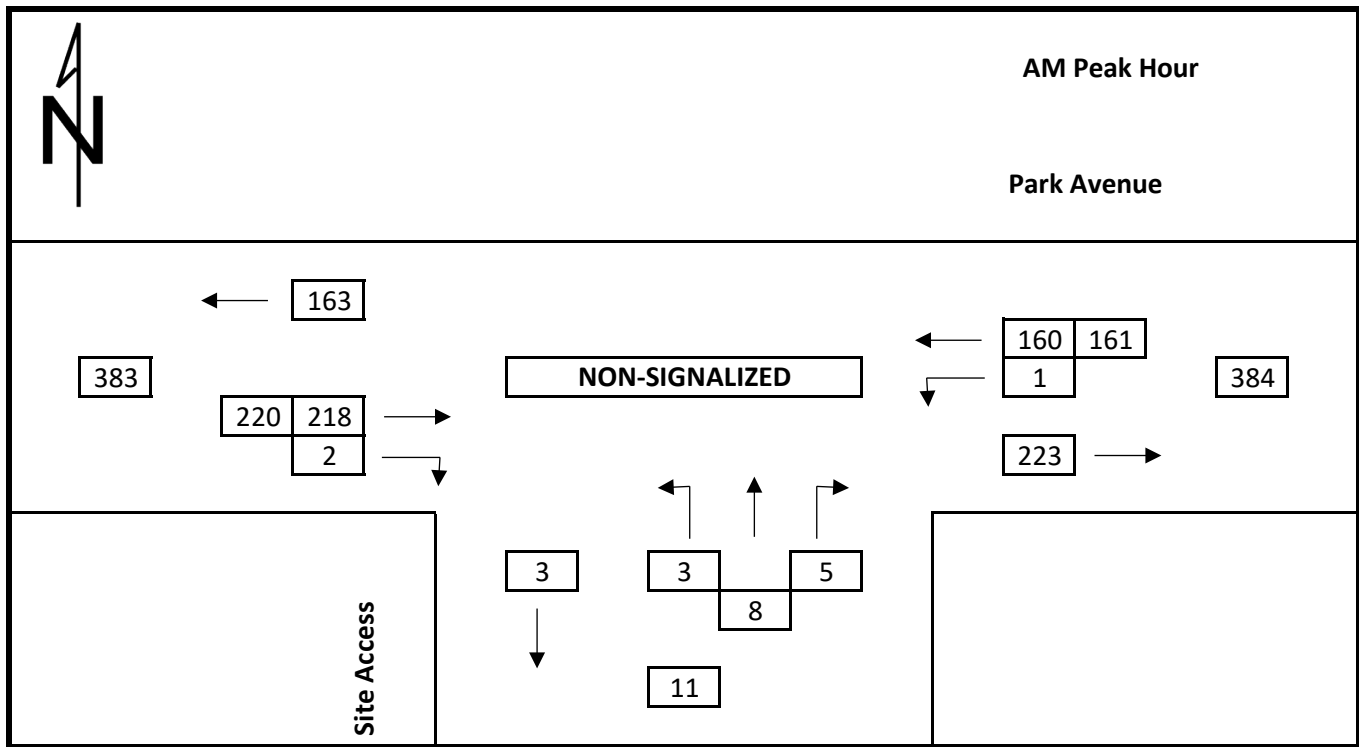




**Total Traffic 2026**  
 Site Access at Park Avenue West



**Total Traffic 2031**  
 Site Access at Park Avenue West


























## **Appendix D**

# **DETAILED SYNCHRO RESULTS**

**Richmond Street at Keil Drive South**  
**Park Avenue West at Keil Drive South**  
**Site Access at Keil Drive South**  
**Site Access at Park Avenue West**

Richmond Street at Keil Drive South  
Chatham, Ontario

Existing Traffic, AM Peak  
Existing Geometric Configuration

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	217	256	57	31	199	130	76	274	45	124	184	202
Future Volume (vph)	217	256	57	31	199	130	76	274	45	124	184	202
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (m)	200.0		0.0	200.0		10.0	100.0		0.0	100.0		10.0
Storage Lanes	1		0	1		1	1		0	1		1
Taper Length (m)	100.0			100.0			100.0			100.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	0.95	1.00
Frt		0.973				0.850		0.979				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1583	3024	0	1630	3107	1444	1583	3183	0	1583	3292	1390
Flt Permitted	0.526			0.547			0.626			0.457		
Satd. Flow (perm)	877	3024	0	938	3107	1444	1043	3183	0	762	3292	1390
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		41				176		28				220
Link Speed (k/h)		60			50			60				50
Link Distance (m)		242.3			224.5			764.8				233.5
Travel Time (s)		14.5			16.2			45.9				16.8
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	5%	7%	7%	2%	7%	3%	5%	2%	4%	5%	1%	7%
Adj. Flow (vph)	236	278	62	34	216	141	83	298	49	135	200	220
Shared Lane Traffic (%)												
Lane Group Flow (vph)	236	340	0	34	216	141	83	347	0	135	200	220
Turn Type	pm+pt	NA		pm+pt	NA	Perm	pm+pt	NA		pm+pt	NA	Perm
Protected Phases	5	2		1	6		7	4		3	8	
Permitted Phases	2			6		6	4			8		8
Detector Phase	5	2		1	6	6	7	4		3	8	8
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	22.5		9.5	22.5	22.5	9.5	22.5		9.5	22.5	22.5
Total Split (s)	10.0	23.5		9.5	23.0	23.0	9.5	22.5		9.5	22.5	22.5
Total Split (%)	15.4%	36.2%		14.6%	35.4%	35.4%	14.6%	34.6%		14.6%	34.6%	34.6%
Maximum Green (s)	5.5	19.0		5.0	18.5	18.5	5.0	18.0		5.0	18.0	18.0
Yellow Time (s)	3.5	3.5		3.5	3.5	3.5	3.5	3.5		3.5	3.5	3.5
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5		4.5	4.5	4.5	4.5	4.5		4.5	4.5	4.5
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag		Lead	Lag	Lag
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	Max	Max		None	Max	Max	None	None		None	None	None
Walk Time (s)		7.0			7.0	7.0		7.0			7.0	7.0
Flash Dont Walk (s)		11.0			11.0	11.0		11.0			11.0	11.0
Pedestrian Calls (#/hr)		0			0	0		0			0	0
Act Effct Green (s)	27.4	25.3		23.8	18.7	18.7	14.7	10.9		15.6	12.8	12.8
Actuated g/C Ratio	0.49	0.45		0.42	0.33	0.33	0.26	0.19		0.28	0.23	0.23
v/c Ratio	0.47	0.24		0.07	0.21	0.24	0.26	0.54		0.47	0.27	0.45
Control Delay	14.2	11.7		9.2	15.6	3.1	14.9	22.4		19.6	19.8	6.9
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0

Richmond Street at Keil Drive South  
Chatham, Ontario

Existing Traffic, AM Peak  
Existing Geometric Configuration

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	14.2	11.7		9.2	15.6	3.1	14.9	22.4		19.6	19.8	6.9
LOS	B	B		A	B	A	B	C		B	B	A
Approach Delay		12.7			10.6			21.0			14.6	
Approach LOS		B			B			C			B	
Queue Length 50th (m)	14.5	9.1		1.8	9.1	0.0	6.2	17.0		10.3	10.2	0.0
Queue Length 95th (m)	30.9	24.2		6.2	17.7	7.5	14.0	28.1		21.1	18.2	14.8
Internal Link Dist (m)		218.3			200.5			740.8			209.5	
Turn Bay Length (m)	200.0			200.0		10.0	100.0			100.0		10.0
Base Capacity (vph)	498	1388		460	1037	599	322	1053		285	1082	604
Starvation Cap Reductn	0	0		0	0	0	0	0		0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	0
Reduced v/c Ratio	0.47	0.24		0.07	0.21	0.24	0.26	0.33		0.47	0.18	0.36
























Intersection Summary	
Area Type:	Other
Cycle Length:	65
Actuated Cycle Length:	56.1
Natural Cycle:	65
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.54
Intersection Signal Delay:	14.6
Intersection LOS:	B
Intersection Capacity Utilization	51.3%
ICU Level of Service	A
Analysis Period (min)	15

Splits and Phases: 6: Keil & Richmond

Ø1	Ø2	Ø3	Ø4
9.5 s	23.5 s	9.5 s	22.5 s
Ø5	Ø6	Ø7	Ø8
10 s	23 s	9.5 s	22.5 s

Richmond Street at Keil Drive South  
Chatham, Ontario

Existing Traffic, PM Peak  
Existing Geometric Configuration

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	299	297	120	46	232	197	64	336	63	131	386	240
Future Volume (vph)	299	297	120	46	232	197	64	336	63	131	386	240
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (m)	200.0		0.0	200.0		10.0	100.0		0.0	100.0		10.0
Storage Lanes	1		0	1		1	1		0	1		1
Taper Length (m)	100.0			100.0			100.0			100.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	0.95	1.00
Frt		0.957				0.850		0.976				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1583	2974	0	1630	3107	1444	1583	3172	0	1583	3292	1390
Flt Permitted	0.525			0.491			0.466			0.384		
Satd. Flow (perm)	875	2974	0	842	3107	1444	777	3172	0	640	3292	1390
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		94				214		33				204
Link Speed (k/h)		60			50			60				50
Link Distance (m)		242.3			224.5			764.8				233.5
Travel Time (s)		14.5			16.2			45.9				16.8
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	5%	7%	7%	2%	7%	3%	5%	2%	4%	5%	1%	7%
Adj. Flow (vph)	325	323	130	50	252	214	70	365	68	142	420	261
Shared Lane Traffic (%)												
Lane Group Flow (vph)	325	453	0	50	252	214	70	433	0	142	420	261
Turn Type	pm+pt	NA		pm+pt	NA	Perm	pm+pt	NA		pm+pt	NA	Perm
Protected Phases	5	2		1	6		7	4		3	8	
Permitted Phases	2			6		6	4			8		8
Detector Phase	5	2		1	6	6	7	4		3	8	8
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	22.5		9.5	22.5	22.5	9.5	22.5		9.5	22.5	22.5
Total Split (s)	10.0	23.5		9.5	23.0	23.0	9.5	22.5		9.5	22.5	22.5
Total Split (%)	15.4%	36.2%		14.6%	35.4%	35.4%	14.6%	34.6%		14.6%	34.6%	34.6%
Maximum Green (s)	5.5	19.0		5.0	18.5	18.5	5.0	18.0		5.0	18.0	18.0
Yellow Time (s)	3.5	3.5		3.5	3.5	3.5	3.5	3.5		3.5	3.5	3.5
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5		4.5	4.5	4.5	4.5	4.5		4.5	4.5	4.5
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag		Lead	Lag	Lag
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	Max	Max		None	Max	Max	None	None		None	None	None
Walk Time (s)		7.0			7.0	7.0		7.0			7.0	7.0
Flash Dont Walk (s)		11.0			11.0	11.0		11.0			11.0	11.0
Pedestrian Calls (#/hr)		0			0	0		0			0	0
Act Effct Green (s)	26.7	23.6		23.9	18.8	18.8	16.9	13.1		17.8	15.0	15.0
Actuated g/C Ratio	0.46	0.40		0.41	0.32	0.32	0.29	0.22		0.30	0.26	0.26
v/c Ratio	0.70	0.36		0.12	0.25	0.35	0.24	0.59		0.51	0.50	0.51
Control Delay	24.8	13.3		10.7	17.2	5.0	14.1	22.4		20.4	21.3	9.8
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0

Richmond Street at Keil Drive South  
Chatham, Ontario

Existing Traffic, PM Peak  
Existing Geometric Configuration



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	24.8	13.3		10.7	17.2	5.0	14.1	22.4		20.4	21.3	9.8
LOS	C	B		B	B	A	B	C		C	C	A
Approach Delay		18.1			11.5			21.2			17.5	
Approach LOS		B			B			C			B	
Queue Length 50th (m)	23.9	17.0		3.0	11.6	0.0	5.1	21.9		10.9	23.0	5.4
Queue Length 95th (m)	#66.0	32.0		9.0	21.7	14.0	12.0	34.6		21.7	35.5	23.3
Internal Link Dist (m)		218.3			200.5			740.8			209.5	
Turn Bay Length (m)	200.0			200.0		10.0	100.0			100.0		10.0
Base Capacity (vph)	467	1259		412	999	609	294	1015		277	1058	585
Starvation Cap Reductn	0	0		0	0	0	0	0		0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	0
Reduced v/c Ratio	0.70	0.36		0.12	0.25	0.35	0.24	0.43		0.51	0.40	0.45

Intersection Summary

Area Type:	Other
Cycle Length:	65
Actuated Cycle Length:	58.4
Natural Cycle:	65
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.70
Intersection Signal Delay:	17.2
Intersection LOS:	B
Intersection Capacity Utilization:	60.1%
ICU Level of Service:	B
Analysis Period (min):	15
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	
























Splits and Phases: 6: Keil & Richmond

9.5 s	23.5 s	9.5 s	22.5 s
10 s	23 s	9.5 s	22.5 s

Richmond Street at Keil Drive South  
Chatham, Ontario

Existing + Site Generated Traffic, AM Peak

Existing Geometric Configuration

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	217	256	58	31	199	130	78	283	47	124	187	202
Future Volume (vph)	217	256	58	31	199	130	78	283	47	124	187	202
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (m)	200.0		0.0	200.0		10.0	100.0		0.0	100.0		10.0
Storage Lanes	1		0	1		1	1		0	1		1
Taper Length (m)	100.0			100.0			100.0			100.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	0.95	1.00
Frt		0.972				0.850		0.979				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1583	3020	0	1630	3107	1444	1583	3182	0	1583	3292	1390
Flt Permitted	0.527			0.547			0.624			0.476		
Satd. Flow (perm)	878	3020	0	938	3107	1444	1040	3182	0	793	3292	1390
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		43				176		28				220
Link Speed (k/h)		60			50			60				50
Link Distance (m)		242.3			224.5			764.8				233.5
Travel Time (s)		14.5			16.2			45.9				16.8
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	5%	7%	7%	2%	7%	3%	5%	2%	4%	5%	1%	7%
Adj. Flow (vph)	236	278	63	34	216	141	85	308	51	135	203	220
Shared Lane Traffic (%)												
Lane Group Flow (vph)	236	341	0	34	216	141	85	359	0	135	203	220
Turn Type	pm+pt	NA		pm+pt	NA	Perm	pm+pt	NA		pm+pt	NA	Perm
Protected Phases	5	2		1	6		7	4		3	8	
Permitted Phases	2			6		6	4			8		8
Detector Phase	5	2		1	6	6	7	4		3	8	8
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	22.5		9.5	22.5	22.5	9.5	22.5		9.5	22.5	22.5
Total Split (s)	10.0	23.5		9.5	23.0	23.0	9.5	22.5		9.5	22.5	22.5
Total Split (%)	15.4%	36.2%		14.6%	35.4%	35.4%	14.6%	34.6%		14.6%	34.6%	34.6%
Maximum Green (s)	5.5	19.0		5.0	18.5	18.5	5.0	18.0		5.0	18.0	18.0
Yellow Time (s)	3.5	3.5		3.5	3.5	3.5	3.5	3.5		3.5	3.5	3.5
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5		4.5	4.5	4.5	4.5	4.5		4.5	4.5	4.5
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag		Lead	Lag	Lag
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	Max	Max		None	Max	Max	None	None		None	None	None
Walk Time (s)		7.0			7.0	7.0		7.0			7.0	7.0
Flash Dont Walk (s)		11.0			11.0	11.0		11.0			11.0	11.0
Pedestrian Calls (#/hr)		0			0	0		0			0	0
Act Effct Green (s)	27.4	25.3		23.8	18.7	18.7	14.9	11.1		14.9	11.1	11.1
Actuated g/C Ratio	0.49	0.45		0.42	0.33	0.33	0.26	0.20		0.26	0.20	0.20
v/c Ratio	0.48	0.25		0.07	0.21	0.24	0.26	0.55		0.48	0.31	0.49
Control Delay	14.4	11.7		9.3	15.8	3.1	14.9	22.6		19.9	21.1	7.6
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0



Richmond Street at Keil Drive South  
Chatham, Ontario

Existing + Site Generated Traffic, AM Peak

Existing Geometric Configuration



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	14.4	11.7		9.3	15.8	3.1	14.9	22.6		19.9	21.1	7.6
LOS	B	B		A	B	A	B	C		B	C	A
Approach Delay		12.8			10.7			21.1			15.5	
Approach LOS		B			B			C			B	
Queue Length 50th (m)	14.7	9.2		1.9	9.1	0.0	6.3	17.7		10.3	10.3	0.0
Queue Length 95th (m)	31.3	24.3		6.2	17.8	7.5	14.3	29.1		21.1	18.4	14.8
Internal Link Dist (m)		218.3			200.5			740.8			209.5	
Turn Bay Length (m)	200.0			200.0		10.0	100.0			100.0		10.0
Base Capacity (vph)	496	1382		458	1033	597	324	1048		281	1065	598
Starvation Cap Reductn	0	0		0	0	0	0	0		0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	0
Reduced v/c Ratio	0.48	0.25		0.07	0.21	0.24	0.26	0.34		0.48	0.19	0.37

Intersection Summary

Area Type:	Other
Cycle Length:	65
Actuated Cycle Length:	56.3
Natural Cycle:	65
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.55
Intersection Signal Delay:	15.0
Intersection LOS:	B
Intersection Capacity Utilization	51.6%
ICU Level of Service	A
Analysis Period (min)	15
























Splits and Phases: 6: Keil & Richmond

9.5 s	23.5 s	9.5 s	22.5 s
10 s	23 s	9.5 s	22.5 s

Richmond Street at Keil Drive South  
Chatham, Ontario

Existing + Site Generated Traffic, PM Peak

Existing Geometric Configuration

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	299	297	124	48	232	197	66	345	64	131	400	240
Future Volume (vph)	299	297	124	48	232	197	66	345	64	131	400	240
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (m)	200.0		0.0	200.0		10.0	100.0		0.0	100.0		10.0
Storage Lanes	1		0	1		1	1		0	1		1
Taper Length (m)	100.0			100.0			100.0			100.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	0.95	1.00
Frt		0.956				0.850		0.976				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1583	2971	0	1630	3107	1444	1583	3172	0	1583	3292	1390
Flt Permitted	0.525			0.488			0.451			0.374		
Satd. Flow (perm)	875	2971	0	837	3107	1444	752	3172	0	623	3292	1390
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		100				214		33				196
Link Speed (k/h)		60			50			60				50
Link Distance (m)		242.3			224.5			764.8				233.5
Travel Time (s)		14.5			16.2			45.9				16.8
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	5%	7%	7%	2%	7%	3%	5%	2%	4%	5%	1%	7%
Adj. Flow (vph)	325	323	135	52	252	214	72	375	70	142	435	261
Shared Lane Traffic (%)												
Lane Group Flow (vph)	325	458	0	52	252	214	72	445	0	142	435	261
Turn Type	pm+pt	NA		pm+pt	NA	Perm	pm+pt	NA		pm+pt	NA	Perm
Protected Phases	5	2		1	6		7	4		3	8	
Permitted Phases	2			6		6	4			8		8
Detector Phase	5	2		1	6	6	7	4		3	8	8
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	22.5		9.5	22.5	22.5	9.5	22.5		9.5	22.5	22.5
Total Split (s)	10.0	23.5		9.5	23.0	23.0	9.5	22.5		9.5	22.5	22.5
Total Split (%)	15.4%	36.2%		14.6%	35.4%	35.4%	14.6%	34.6%		14.6%	34.6%	34.6%
Maximum Green (s)	5.5	19.0		5.0	18.5	18.5	5.0	18.0		5.0	18.0	18.0
Yellow Time (s)	3.5	3.5		3.5	3.5	3.5	3.5	3.5		3.5	3.5	3.5
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5		4.5	4.5	4.5	4.5	4.5		4.5	4.5	4.5
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag		Lead	Lag	Lag
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	Max	Max		None	Max	Max	None	None		None	None	None
Walk Time (s)		7.0			7.0	7.0		7.0			7.0	7.0
Flash Dont Walk (s)		11.0			11.0	11.0		11.0			11.0	11.0
Pedestrian Calls (#/hr)		0			0	0		0			0	0
Act Effct Green (s)	26.7	23.6		23.9	18.8	18.8	17.1	13.3		18.0	15.2	15.2
Actuated g/C Ratio	0.46	0.40		0.41	0.32	0.32	0.29	0.23		0.31	0.26	0.26
v/c Ratio	0.70	0.36		0.13	0.25	0.35	0.25	0.60		0.52	0.51	0.52
Control Delay	25.0	13.2		10.9	17.3	5.0	14.2	22.5		20.6	21.4	10.3
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0

Richmond Street at Keil Drive South  
Chatham, Ontario

Existing + Site Generated Traffic, PM Peak  
Existing Geometric Configuration



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	25.0	13.2		10.9	17.3	5.0	14.2	22.5		20.6	21.4	10.3
LOS	C	B		B	B	A	B	C		C	C	B
Approach Delay		18.1			11.6			21.4			17.8	
Approach LOS		B			B			C			B	
Queue Length 50th (m)	24.1	17.1		3.2	11.7	0.0	5.3	22.7		10.9	24.0	6.2
Queue Length 95th (m)	#66.0	31.9		9.3	21.7	14.0	12.3	35.6		21.7	36.7	24.5
Internal Link Dist (m)		218.3			200.5			740.8			209.5	
Turn Bay Length (m)	200.0			200.0		10.0	100.0			100.0		10.0
Base Capacity (vph)	466	1257		409	995	608	291	1012		274	1058	579
Starvation Cap Reductn	0	0		0	0	0	0	0		0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	0
Reduced v/c Ratio	0.70	0.36		0.13	0.25	0.35	0.25	0.44		0.52	0.41	0.45

Intersection Summary
























Area Type:	Other
Cycle Length:	65
Actuated Cycle Length:	58.6
Natural Cycle:	65
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.70
Intersection Signal Delay:	17.4
Intersection LOS:	B
Intersection Capacity Utilization:	60.4%
ICU Level of Service:	B
Analysis Period (min):	15
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	

Splits and Phases: 6: Keil & Richmond

Ø1	Ø2	Ø3	Ø4
9.5 s	23.5 s	9.5 s	22.5 s
Ø5	Ø6	Ø7	Ø8
10 s	23 s	9.5 s	22.5 s

Richmond Street at Keil Drive South  
Chatham, Ontario

Total Traffic 2026, AM Peak  
Existing Geometric Configuration

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	230	272	62	33	211	138	83	300	50	132	198	214
Future Volume (vph)	230	272	62	33	211	138	83	300	50	132	198	214
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (m)	200.0		0.0	200.0		10.0	100.0		0.0	100.0		10.0
Storage Lanes	1		0	1		1	1		0	1		1
Taper Length (m)	100.0			100.0			100.0			100.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	0.95	1.00
Frt		0.972				0.850		0.979				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1583	3020	0	1630	3107	1444	1583	3182	0	1583	3292	1390
Flt Permitted	0.520			0.535			0.617			0.452		
Satd. Flow (perm)	867	3020	0	918	3107	1444	1028	3182	0	753	3292	1390
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		42				176		28				233
Link Speed (k/h)		60			50			60				50
Link Distance (m)		242.3			224.5			764.8				233.5
Travel Time (s)		14.5			16.2			45.9				16.8
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	5%	7%	7%	2%	7%	3%	5%	2%	4%	5%	1%	7%
Adj. Flow (vph)	250	296	67	36	229	150	90	326	54	143	215	233
Shared Lane Traffic (%)												
Lane Group Flow (vph)	250	363	0	36	229	150	90	380	0	143	215	233
Turn Type	pm+pt	NA		pm+pt	NA	Perm	pm+pt	NA		pm+pt	NA	Perm
Protected Phases	5	2		1	6		7	4		3	8	
Permitted Phases	2			6		6	4			8		8
Detector Phase	5	2		1	6	6	7	4		3	8	8
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	22.5		9.5	22.5	22.5	9.5	22.5		9.5	22.5	22.5
Total Split (s)	10.0	23.5		9.5	23.0	23.0	9.5	22.5		9.5	22.5	22.5
Total Split (%)	15.4%	36.2%		14.6%	35.4%	35.4%	14.6%	34.6%		14.6%	34.6%	34.6%
Maximum Green (s)	5.5	19.0		5.0	18.5	18.5	5.0	18.0		5.0	18.0	18.0
Yellow Time (s)	3.5	3.5		3.5	3.5	3.5	3.5	3.5		3.5	3.5	3.5
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5		4.5	4.5	4.5	4.5	4.5		4.5	4.5	4.5
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag		Lead	Lag	Lag
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	Max	Max		None	Max	Max	None	None		None	None	None
Walk Time (s)		7.0			7.0	7.0		7.0			7.0	7.0
Flash Dont Walk (s)		11.0			11.0	11.0		11.0			11.0	11.0
Pedestrian Calls (#/hr)		0			0	0		0			0	0
Act Effct Green (s)	27.4	25.4		23.8	18.8	18.8	15.4	11.5		15.4	11.5	11.5
Actuated g/C Ratio	0.48	0.45		0.42	0.33	0.33	0.27	0.20		0.27	0.20	0.20
v/c Ratio	0.51	0.26		0.08	0.22	0.25	0.28	0.57		0.52	0.32	0.50
Control Delay	15.6	12.1		9.6	16.1	3.6	15.0	22.8		20.9	21.1	7.4
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0

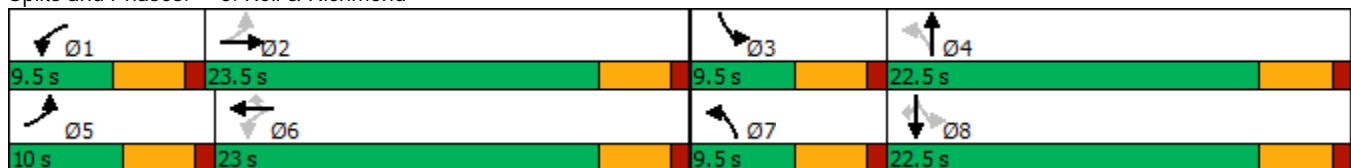
Richmond Street at Keil Drive South  
Chatham, Ontario

Total Traffic 2026, AM Peak  
Existing Geometric Configuration

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	15.6	12.1		9.6	16.1	3.6	15.0	22.8		20.9	21.1	7.4
LOS	B	B		A	B	A	B	C		C	C	A
Approach Delay		13.5			11.0			21.3			15.7	
Approach LOS		B			B			C			B	
Queue Length 50th (m)	16.0	10.1		2.0	9.8	0.0	6.7	19.0		11.0	11.0	0.0
Queue Length 95th (m)	33.7	26.3		6.7	19.0	8.8	14.9	30.7		22.1	19.2	15.1
Internal Link Dist (m)		218.3			200.5			740.8			209.5	
Turn Bay Length (m)	200.0			200.0		10.0	100.0			100.0		10.0
Base Capacity (vph)	488	1371		448	1025	594	327	1041		277	1057	604
Starvation Cap Reductn	0	0		0	0	0	0	0		0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	0
Reduced v/c Ratio	0.51	0.26		0.08	0.22	0.25	0.28	0.37		0.52	0.20	0.39























Intersection Summary	
Area Type:	Other
Cycle Length:	65
Actuated Cycle Length:	56.8
Natural Cycle:	65
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.57
Intersection Signal Delay:	15.4
Intersection LOS:	B
Intersection Capacity Utilization	53.8%
ICU Level of Service	A
Analysis Period (min)	15

Splits and Phases: 6: Keil & Richmond



Richmond Street at Keil Drive South  
Chatham, Ontario

Total Traffic 2026, PM Peak  
Existing Geometric Configuration

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	317	315	131	51	246	209	70	366	68	139	424	255
Future Volume (vph)	317	315	131	51	246	209	70	366	68	139	424	255
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (m)	200.0		0.0	200.0		10.0	100.0		0.0	100.0		10.0
Storage Lanes	1		0	1		1	1		0	1		1
Taper Length (m)	100.0			100.0			100.0			100.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	0.95	1.00
Frt		0.956				0.850		0.976				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1583	2971	0	1630	3107	1444	1583	3172	0	1583	3292	1390
Flt Permitted	0.517			0.476			0.425			0.351		
Satd. Flow (perm)	862	2971	0	817	3107	1444	708	3172	0	585	3292	1390
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		98				227		33				196
Link Speed (k/h)		60			50			60				50
Link Distance (m)		242.3			224.5			764.8				233.5
Travel Time (s)		14.5			16.2			45.9				16.8
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	5%	7%	7%	2%	7%	3%	5%	2%	4%	5%	1%	7%
Adj. Flow (vph)	345	342	142	55	267	227	76	398	74	151	461	277
Shared Lane Traffic (%)												
Lane Group Flow (vph)	345	484	0	55	267	227	76	472	0	151	461	277
Turn Type	pm+pt	NA		pm+pt	NA	Perm	pm+pt	NA		pm+pt	NA	Perm
Protected Phases	5	2		1	6		7	4		3	8	
Permitted Phases	2			6		6	4			8		8
Detector Phase	5	2		1	6	6	7	4		3	8	8
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	22.5		9.5	22.5	22.5	9.5	22.5		9.5	22.5	22.5
Total Split (s)	10.0	23.5		9.5	23.0	23.0	9.5	22.5		9.5	22.5	22.5
Total Split (%)	15.4%	36.2%		14.6%	35.4%	35.4%	14.6%	34.6%		14.6%	34.6%	34.6%
Maximum Green (s)	5.5	19.0		5.0	18.5	18.5	5.0	18.0		5.0	18.0	18.0
Yellow Time (s)	3.5	3.5		3.5	3.5	3.5	3.5	3.5		3.5	3.5	3.5
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5		4.5	4.5	4.5	4.5	4.5		4.5	4.5	4.5
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag		Lead	Lag	Lag
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	Max	Max		None	Max	Max	None	None		None	None	None
Walk Time (s)		7.0			7.0	7.0		7.0			7.0	7.0
Flash Dont Walk (s)		11.0			11.0	11.0		11.0			11.0	11.0
Pedestrian Calls (#/hr)		0			0	0		0			0	0
Act Effct Green (s)	26.7	23.6		23.9	18.8	18.8	17.5	13.7		18.4	15.6	15.6
Actuated g/C Ratio	0.45	0.40		0.41	0.32	0.32	0.30	0.23		0.31	0.26	0.26
v/c Ratio	0.75	0.39		0.14	0.27	0.37	0.27	0.62		0.56	0.53	0.54
Control Delay	29.0	13.8		11.1	17.6	5.1	14.5	23.0		22.4	21.6	11.3
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0

Richmond Street at Keil Drive South  
Chatham, Ontario

Total Traffic 2026, PM Peak  
Existing Geometric Configuration

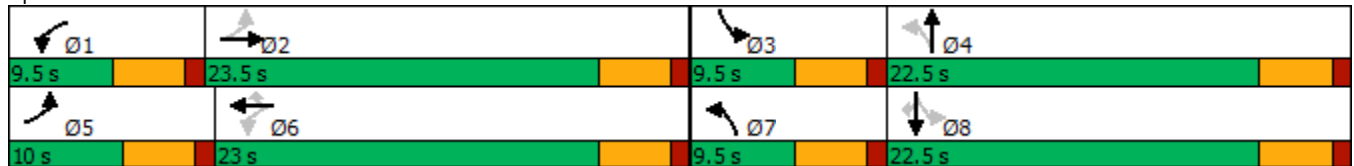


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	29.0	13.8		11.1	17.6	5.1	14.5	23.0		22.4	21.6	11.3
LOS	C	B		B	B	A	B	C		C	C	B
Approach Delay		20.1			11.8			21.8			18.6	
Approach LOS		C			B			C			B	
Queue Length 50th (m)	26.5	18.9		3.4	12.6	0.0	5.6	24.4		11.7	25.6	7.8
Queue Length 95th (m)	#74.8	34.3		9.7	23.0	14.3	12.9	38.0		22.9	39.0	27.6
Internal Link Dist (m)		218.3			200.5			740.8			209.5	
Turn Bay Length (m)	200.0			200.0		10.0	100.0			100.0		10.0
Base Capacity (vph)	458	1248		400	989	614	285	1005		268	1055	579
Starvation Cap Reductn	0	0		0	0	0	0	0		0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	0
Reduced v/c Ratio	0.75	0.39		0.14	0.27	0.37	0.27	0.47		0.56	0.44	0.48

Intersection Summary
























Area Type:	Other
Cycle Length:	65
Actuated Cycle Length:	59
Natural Cycle:	65
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.75
Intersection Signal Delay:	18.3
Intersection LOS:	B
Intersection Capacity Utilization:	63.2%
ICU Level of Service:	B
Analysis Period (min):	15
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	

Splits and Phases: 6: Keil & Richmond



Richmond Street at Keil Drive South  
Chatham, Ontario

Total Traffic 2031, AM Peak  
Existing Geometric Configuration

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	242	286	65	35	222	145	87	315	52	138	208	225
Future Volume (vph)	242	286	65	35	222	145	87	315	52	138	208	225
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (m)	200.0		0.0	200.0		10.0	100.0		0.0	100.0		10.0
Storage Lanes	1		0	1		1	1		0	1		1
Taper Length (m)	100.0			100.0			100.0			100.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	0.95	1.00
Frt		0.972				0.850		0.979				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1583	3020	0	1630	3107	1444	1583	3182	0	1583	3292	1390
Flt Permitted	0.507			0.525			0.611			0.431		
Satd. Flow (perm)	845	3020	0	901	3107	1444	1018	3182	0	718	3292	1390
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		43				176		29				245
Link Speed (k/h)		60			50			60				50
Link Distance (m)		242.3			224.5			764.8				233.5
Travel Time (s)		14.5			16.2			45.9				16.8
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	5%	7%	7%	2%	7%	3%	5%	2%	4%	5%	1%	7%
Adj. Flow (vph)	263	311	71	38	241	158	95	342	57	150	226	245
Shared Lane Traffic (%)												
Lane Group Flow (vph)	263	382	0	38	241	158	95	399	0	150	226	245
Turn Type	pm+pt	NA		pm+pt	NA	Perm	pm+pt	NA		pm+pt	NA	Perm
Protected Phases	5	2		1	6		7	4		3	8	
Permitted Phases	2			6		6	4			8		8
Detector Phase	5	2		1	6	6	7	4		3	8	8
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	22.5		9.5	22.5	22.5	9.5	22.5		9.5	22.5	22.5
Total Split (s)	10.2	23.5		9.5	22.8	22.8	9.5	22.5		9.5	22.5	22.5
Total Split (%)	15.7%	36.2%		14.6%	35.1%	35.1%	14.6%	34.6%		14.6%	34.6%	34.6%
Maximum Green (s)	5.7	19.0		5.0	18.3	18.3	5.0	18.0		5.0	18.0	18.0
Yellow Time (s)	3.5	3.5		3.5	3.5	3.5	3.5	3.5		3.5	3.5	3.5
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5		4.5	4.5	4.5	4.5	4.5		4.5	4.5	4.5
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag		Lead	Lag	Lag
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	Max	Max		None	Max	Max	None	None		None	None	None
Walk Time (s)		7.0			7.0	7.0		7.0			7.0	7.0
Flash Dont Walk (s)		11.0			11.0	11.0		11.0			11.0	11.0
Pedestrian Calls (#/hr)		0			0	0		0			0	0
Act Effct Green (s)	27.5	25.4		23.6	18.5	18.5	15.7	11.9		15.7	11.9	11.9
Actuated g/C Ratio	0.48	0.44		0.41	0.32	0.32	0.27	0.21		0.27	0.21	0.21
v/c Ratio	0.55	0.28		0.09	0.24	0.27	0.29	0.58		0.55	0.33	0.51
Control Delay	17.1	12.4		9.8	16.5	4.1	15.1	22.9		22.1	21.0	7.3
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0



Richmond Street at Keil Drive South  
Chatham, Ontario

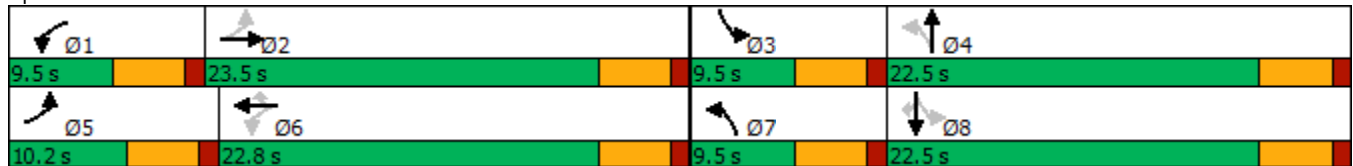
Total Traffic 2031, AM Peak  
Existing Geometric Configuration

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	17.1	12.4		9.8	16.5	4.1	15.1	22.9		22.1	21.0	7.3
LOS	B	B		A	B	A	B	C		C	C	A
Approach Delay		14.3			11.5			21.4			15.9	
Approach LOS		B			B			C			B	
Queue Length 50th (m)	17.2	10.9		2.1	10.6	0.0	7.1	20.1		11.6	11.6	0.0
Queue Length 95th (m)	#39.6	28.1		7.1	20.3	10.0	15.5	32.2		23.0	20.1	15.3
Internal Link Dist (m)		218.3			200.5			740.8			209.5	
Turn Bay Length (m)	200.0			200.0		10.0	100.0			100.0		10.0
Base Capacity (vph)	480	1365		436	1008	587	330	1035		274	1051	610
Starvation Cap Reductn	0	0		0	0	0	0	0		0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	0
Reduced v/c Ratio	0.55	0.28		0.09	0.24	0.27	0.29	0.39		0.55	0.22	0.40

Intersection Summary























Area Type: Other  
 Cycle Length: 65  
 Actuated Cycle Length: 57.1  
 Natural Cycle: 65  
 Control Type: Semi Act-Uncoord  
 Maximum v/c Ratio: 0.58  
 Intersection Signal Delay: 15.8  
 Intersection LOS: B  
 Intersection Capacity Utilization 55.8%  
 ICU Level of Service B  
 Analysis Period (min) 15  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

Splits and Phases: 6: Keil & Richmond



Richmond Street at Keil Drive South  
Chatham, Ontario

Total Traffic 2031, PM Peak  
Existing Geometric Configuration

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	334	331	138	53	259	220	73	384	71	146	445	268
Future Volume (vph)	334	331	138	53	259	220	73	384	71	146	445	268
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (m)	200.0		0.0	200.0		10.0	100.0		0.0	100.0		10.0
Storage Lanes	1		0	1		1	1		0	1		1
Taper Length (m)	100.0			100.0			100.0			100.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95	1.00	0.95	1.00
Frt		0.956				0.850		0.977				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1583	2971	0	1630	3107	1444	1583	3175	0	1583	3292	1390
Flt Permitted	0.474			0.464			0.385			0.310		
Satd. Flow (perm)	790	2971	0	796	3107	1444	642	3175	0	517	3292	1390
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		99				239		29				234
Link Speed (k/h)		60			50			60				50
Link Distance (m)		242.3			224.5			764.8				233.5
Travel Time (s)		14.5			16.2			45.9				16.8
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	5%	7%	7%	2%	7%	3%	5%	2%	4%	5%	1%	7%
Adj. Flow (vph)	363	360	150	58	282	239	79	417	77	159	484	291
Shared Lane Traffic (%)												
Lane Group Flow (vph)	363	510	0	58	282	239	79	494	0	159	484	291
Turn Type	pm+pt	NA		pm+pt	NA	Perm	pm+pt	NA		pm+pt	NA	Perm
Protected Phases	5	2		1	6		7	4		3	8	
Permitted Phases	2			6		6	4			8		8
Detector Phase	5	2		1	6	6	7	4		3	8	8
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	22.5		9.5	22.5	22.5	9.5	22.5		9.5	22.5	22.5
Total Split (s)	15.0	28.3		9.6	22.9	22.9	9.5	22.5		9.6	22.6	22.6
Total Split (%)	21.4%	40.4%		13.7%	32.7%	32.7%	13.6%	32.1%		13.7%	32.3%	32.3%
Maximum Green (s)	10.5	23.8		5.1	18.4	18.4	5.0	18.0		5.1	18.1	18.1
Yellow Time (s)	3.5	3.5		3.5	3.5	3.5	3.5	3.5		3.5	3.5	3.5
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5		4.5	4.5	4.5	4.5	4.5		4.5	4.5	4.5
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag		Lead	Lag	Lag
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	Max	Max		None	Max	Max	None	None		None	None	None
Walk Time (s)		7.0			7.0	7.0		7.0			7.0	7.0
Flash Dont Walk (s)		11.0			11.0	11.0		11.0			11.0	11.0
Pedestrian Calls (#/hr)		0			0	0		0			0	0
Act Effct Green (s)	33.5	27.9		23.6	18.4	18.4	19.9	14.9		21.0	17.0	17.0
Actuated g/C Ratio	0.50	0.42		0.35	0.27	0.27	0.30	0.22		0.31	0.25	0.25
v/c Ratio	0.70	0.39		0.17	0.33	0.42	0.30	0.68		0.65	0.58	0.55
Control Delay	21.1	13.5		11.6	21.3	5.8	17.3	27.5		30.8	25.7	10.3
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0

Richmond Street at Keil Drive South  
Chatham, Ontario

Total Traffic 2031, PM Peak  
Existing Geometric Configuration

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	21.1	13.5		11.6	21.3	5.8	17.3	27.5		30.8	25.7	10.3
LOS	C	B		B	C	A	B	C		C	C	B
Approach Delay		16.7			13.9			26.1			21.8	
Approach LOS		B			B			C			C	
Queue Length 50th (m)	29.6	21.2		3.8	15.7	0.0	6.7	29.4		14.3	30.4	6.1
Queue Length 95th (m)	#60.6	35.9		9.9	26.7	15.7	15.0	44.3		#31.7	45.1	26.7
Internal Link Dist (m)		218.3			200.5			740.8			209.5	
Turn Bay Length (m)	200.0			200.0		10.0	100.0			100.0		10.0
Base Capacity (vph)	519	1294		343	855	570	260	876		243	911	554
Starvation Cap Reductn	0	0		0	0	0	0	0		0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	0
Reduced v/c Ratio	0.70	0.39		0.17	0.33	0.42	0.30	0.56		0.65	0.53	0.53

**Intersection Summary**

Area Type: Other

Cycle Length: 70

Actuated Cycle Length: 67

Natural Cycle: 70

Control Type: Semi Act-Uncoord

Maximum v/c Ratio: 0.70

Intersection Signal Delay: 19.6

Intersection LOS: B

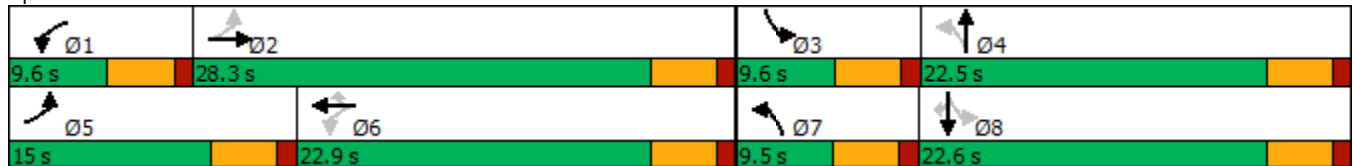
Intersection Capacity Utilization 65.6%

ICU Level of Service C

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.

Splits and Phases: 6: Keil & Richmond



Park Avenue West at Keil Drive South  
Chatham, Ontario

Existing Traffic, AM Peak  
Existing Geometric Configuration

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	55	106	34	27	77	205	22	141	30	150	91	44
Future Volume (vph)	55	106	34	27	77	205	22	141	30	150	91	44
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (m)	50.0		0.0	40.0		0.0	35.0		0.0	110.0		0.0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (m)	55.0			40.0			40.0			80.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	0.95	0.95
Frt		0.963			0.891			0.973			0.951	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1614	3008	0	1583	2841	0	1614	1683	0	1583	3111	0
Flt Permitted	0.528			0.656			0.659			0.517		
Satd. Flow (perm)	897	3008	0	1093	2841	0	1120	1683	0	862	3111	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		37			223			15			48	
Link Speed (k/h)		60			60			60			60	
Link Distance (m)		117.3			179.0			91.1			764.8	
Travel Time (s)		7.0			10.7			5.5			45.9	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	5%	11%	5%	5%	4%	3%	1%	2%	5%	1%	3%
Adj. Flow (vph)	60	115	37	29	84	223	24	153	33	163	99	48
Shared Lane Traffic (%)												
Lane Group Flow (vph)	60	152	0	29	307	0	24	186	0	163	147	0
Turn Type	pm+pt	NA		pm+pt	NA		pm+pt	NA		pm+pt	NA	
Protected Phases	5	2		1	6		7	4		3	8	
Permitted Phases	2			6			4			8		
Detector Phase	5	2		1	6		7	4		3	8	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	10.0	24.0		9.5	24.0		9.5	24.0		9.5	24.0	
Total Split (s)	10.0	26.1		9.5	25.6		9.5	24.0		10.4	24.9	
Total Split (%)	14.3%	37.3%		13.6%	36.6%		13.6%	34.3%		14.9%	35.6%	
Maximum Green (s)	7.0	20.1		6.5	19.6		6.5	18.0		7.4	18.9	
Yellow Time (s)	3.0	4.0		3.0	4.0		3.0	4.0		3.0	4.0	
All-Red Time (s)	0.0	2.0		0.0	2.0		0.0	2.0		0.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	3.0	6.0		3.0	6.0		3.0	6.0		3.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag		Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	Max		None	Max		None	None		None	None	
Walk Time (s)		7.0			7.0			7.0			7.0	
Flash Dont Walk (s)		11.0			11.0			11.0			11.0	
Pedestrian Calls (#/hr)		0			0			0			0	
Act Effct Green (s)	28.7	23.1		27.6	21.1		19.1	11.4		21.6	16.0	
Actuated g/C Ratio	0.49	0.40		0.48	0.36		0.33	0.20		0.37	0.28	
v/c Ratio	0.11	0.12		0.05	0.26		0.06	0.55		0.40	0.17	
Control Delay	9.8	12.4		9.4	6.5		11.8	27.1		15.4	13.6	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	

Park Avenue West at Keil Drive South  
Chatham, Ontario

Existing Traffic, AM Peak  
Existing Geometric Configuration

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	9.8	12.4		9.4	6.5		11.8	27.1		15.4	13.6	
LOS	A	B		A	A		B	C		B	B	
Approach Delay		11.6			6.8			25.4			14.6	
Approach LOS		B			A			C			B	
Queue Length 50th (m)	3.5	3.9		1.7	3.8		1.8	19.5		13.2	4.2	
Queue Length 95th (m)	10.2	12.6		6.0	13.4		5.6	37.3		25.0	12.1	
Internal Link Dist (m)		93.3			155.0			67.1			740.8	
Turn Bay Length (m)	50.0			40.0			35.0			110.0		
Base Capacity (vph)	532	1219		577	1174		431	552		416	1119	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.11	0.12		0.05	0.26		0.06	0.34		0.39	0.13	

Intersection Summary

Area Type:	Other
Cycle Length:	70
Actuated Cycle Length:	58.1
Natural Cycle:	70
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.55
Intersection Signal Delay:	13.7
Intersection Capacity Utilization	49.4%
Analysis Period (min)	15
Intersection LOS:	B
ICU Level of Service	A

Splits and Phases: 3: Keil & Park W

Ø1	Ø2	Ø3	Ø4
9.5 s	26.1 s	10.4 s	24 s
Ø5	Ø6	Ø7	Ø8
10 s	25.6 s	9.5 s	24.9 s

Park Avenue West at Keil Drive South  
Chatham, Ontario

Existing Traffic, PM Peak  
Existing Geometric Configuration

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	69	185	32	14	151	214	12	98	11	282	163	92
Future Volume (vph)	69	185	32	14	151	214	12	98	11	282	163	92
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (m)	50.0		0.0	40.0		0.0	35.0		0.0	110.0		0.0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (m)	55.0			40.0			40.0			80.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	0.95	0.95
Frt		0.978			0.912			0.985			0.946	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1614	3071	0	1583	2904	0	1614	1705	0	1583	3092	0
Flt Permitted	0.459			0.605			0.581			0.501		
Satd. Flow (perm)	780	3071	0	1008	2904	0	987	1705	0	835	3092	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		27			233			8			100	
Link Speed (k/h)		60			60			60			60	
Link Distance (m)		117.3			179.0			91.1			764.8	
Travel Time (s)		7.0			10.7			5.5			45.9	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	5%	11%	5%	5%	4%	3%	1%	2%	5%	1%	3%
Adj. Flow (vph)	75	201	35	15	164	233	13	107	12	307	177	100
Shared Lane Traffic (%)												
Lane Group Flow (vph)	75	236	0	15	397	0	13	119	0	307	277	0
Turn Type	pm+pt	NA		pm+pt	NA		pm+pt	NA		pm+pt	NA	
Protected Phases	5	2		1	6		7	4		3	8	
Permitted Phases	2			6			4			8		
Detector Phase	5	2		1	6		7	4		3	8	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	10.0	24.0		9.5	24.0		9.5	24.0		9.5	24.0	
Total Split (s)	10.0	24.5		9.5	24.0		9.5	24.0		12.0	26.5	
Total Split (%)	14.3%	35.0%		13.6%	34.3%		13.6%	34.3%		17.1%	37.9%	
Maximum Green (s)	7.0	18.5		6.5	18.0		6.5	18.0		9.0	20.5	
Yellow Time (s)	3.0	4.0		3.0	4.0		3.0	4.0		3.0	4.0	
All-Red Time (s)	0.0	2.0		0.0	2.0		0.0	2.0		0.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	3.0	6.0		3.0	6.0		3.0	6.0		3.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag		Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	Max		None	Max		None	None		None	None	
Walk Time (s)		7.0			7.0			7.0			7.0	
Flash Dont Walk (s)		11.0			11.0			11.0			11.0	
Pedestrian Calls (#/hr)		0			0			0			0	
Act Effect Green (s)	27.1	22.8		25.5	19.2		15.9	9.3		21.3	16.6	
Actuated g/C Ratio	0.49	0.41		0.46	0.35		0.29	0.17		0.39	0.30	
v/c Ratio	0.15	0.18		0.03	0.34		0.04	0.41		0.69	0.28	
Control Delay	9.7	11.9		8.9	8.4		11.4	26.0		22.7	11.1	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	

Park Avenue West at Keil Drive South  
Chatham, Ontario

Existing Traffic, PM Peak  
Existing Geometric Configuration

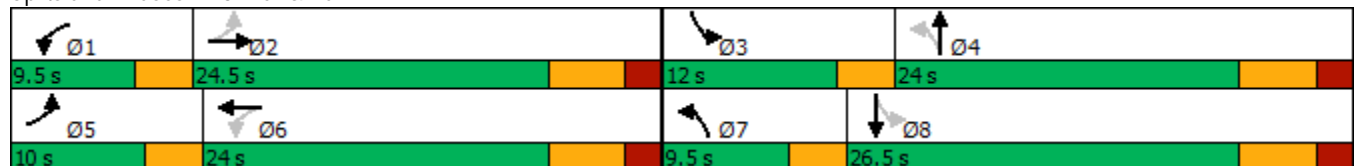


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	9.7	11.9		8.9	8.4		11.4	26.0		22.7	11.1	
LOS	A	B		A	A		B	C		C	B	
Approach Delay		11.4			8.5			24.5				17.2
Approach LOS		B			A			C				B
Queue Length 50th (m)	4.3	7.1		0.8	7.5		0.9	12.2		26.3	7.4	
Queue Length 95th (m)	11.4	19.1		3.6	18.7		3.6	25.7		#47.9	18.9	
Internal Link Dist (m)		93.3			155.0			67.1				740.8
Turn Bay Length (m)	50.0			40.0			35.0			110.0		
Base Capacity (vph)	494	1284		540	1160		373	588		450	1296	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.15	0.18		0.03	0.34		0.03	0.20		0.68	0.21	

Intersection Summary





















Area Type:	Other
Cycle Length:	70
Actuated Cycle Length:	55.2
Natural Cycle:	70
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.69
Intersection Signal Delay:	14.1
Intersection LOS:	B
Intersection Capacity Utilization:	53.1%
ICU Level of Service:	A
Analysis Period (min):	15
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	

Splits and Phases: 3: Keil & Park W



Park Avenue West at Keil Drive South  
Chatham, Ontario

Existing + Site Generated Traffic, AM Peak  
Existing Geometric Configuration

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	57	109	34	28	78	205	22	152	33	150	95	44
Future Volume (vph)	57	109	34	28	78	205	22	152	33	150	95	44
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (m)	50.0		0.0	40.0		0.0	35.0		0.0	110.0		0.0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (m)	55.0			40.0			40.0			80.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	0.95	0.95
Frt		0.964			0.891			0.973			0.952	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1614	3012	0	1583	2841	0	1614	1683	0	1583	3114	0
Flt Permitted	0.527			0.654			0.656			0.510		
Satd. Flow (perm)	895	3012	0	1090	2841	0	1115	1683	0	850	3114	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		37			223			15			48	
Link Speed (k/h)		60			60			60			60	
Link Distance (m)		117.3			179.0			91.1			764.8	
Travel Time (s)		7.0			10.7			5.5			45.9	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	5%	11%	5%	5%	4%	3%	1%	2%	5%	1%	3%
Adj. Flow (vph)	62	118	37	30	85	223	24	165	36	163	103	48
Shared Lane Traffic (%)												
Lane Group Flow (vph)	62	155	0	30	308	0	24	201	0	163	151	0
Turn Type	pm+pt	NA		pm+pt	NA		pm+pt	NA		pm+pt	NA	
Protected Phases	5	2		1	6		7	4		3	8	
Permitted Phases	2			6			4			8		
Detector Phase	5	2		1	6		7	4		3	8	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	10.0	24.0		9.5	24.0		9.5	24.0		9.5	24.0	
Total Split (s)	10.0	26.1		9.5	25.6		9.5	24.0		10.4	24.9	
Total Split (%)	14.3%	37.3%		13.6%	36.6%		13.6%	34.3%		14.9%	35.6%	
Maximum Green (s)	7.0	20.1		6.5	19.6		6.5	18.0		7.4	18.9	
Yellow Time (s)	3.0	4.0		3.0	4.0		3.0	4.0		3.0	4.0	
All-Red Time (s)	0.0	2.0		0.0	2.0		0.0	2.0		0.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	3.0	6.0		3.0	6.0		3.0	6.0		3.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag		Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	Max		None	Max		None	None		None	None	
Walk Time (s)		7.0			7.0			7.0			7.0	
Flash Dont Walk (s)		11.0			11.0			11.0			11.0	
Pedestrian Calls (#/hr)		0			0			0			0	
Act Effct Green (s)	28.4	22.8		27.3	20.9		19.7	12.0		22.2	16.5	
Actuated g/C Ratio	0.49	0.39		0.47	0.36		0.34	0.21		0.38	0.28	
v/c Ratio	0.12	0.13		0.05	0.27		0.06	0.56		0.39	0.17	
Control Delay	10.1	12.7		9.8	6.7		11.6	27.4		15.2	13.5	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	



Park Avenue West at Keil Drive South  
Chatham, Ontario

Existing + Site Generated Traffic, AM Peak  
Existing Geometric Configuration

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	10.1	12.7		9.8	6.7		11.6	27.4		15.2	13.5	
LOS	B	B		A	A		B	C		B	B	
Approach Delay		11.9			7.0			25.7			14.4	
Approach LOS		B			A			C			B	
Queue Length 50th (m)	3.7	4.1		1.8	3.9		1.8	21.4		13.2	4.4	
Queue Length 95th (m)	10.7	13.1		6.4	13.6		5.6	39.9		25.0	12.3	
Internal Link Dist (m)		93.3			155.0			67.1			740.8	
Turn Bay Length (m)	50.0			40.0			35.0			110.0		
Base Capacity (vph)	525	1200		568	1158		439	552		420	1130	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.12	0.13		0.05	0.27		0.05	0.36		0.39	0.13	

Intersection Summary





















Area Type:	Other
Cycle Length:	70
Actuated Cycle Length:	58.4
Natural Cycle:	70
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.56
Intersection Signal Delay:	13.9
Intersection LOS:	B
Intersection Capacity Utilization:	50.2%
ICU Level of Service:	A
Analysis Period (min):	15

Splits and Phases: 3: Keil & Park W

Ø1	Ø2	Ø3	Ø4
9.5 s	26.1 s	10.4 s	24 s
Ø5	Ø6	Ø7	Ø8
10 s	25.6 s	9.5 s	24.9 s

Park Avenue West at Keil Drive South  
Chatham, Ontario

Existing + Site Generated Traffic, PM Peak  
Existing Geometric Configuration

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	70	188	32	15	154	214	12	109	11	282	181	94
Future Volume (vph)	70	188	32	15	154	214	12	109	11	282	181	94
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (m)	50.0		0.0	40.0		0.0	35.0		0.0	110.0		0.0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (m)	55.0			40.0			40.0			80.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	0.95	0.95
Frt		0.978			0.913			0.986			0.949	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1614	3071	0	1583	2907	0	1614	1707	0	1583	3103	0
Flt Permitted	0.458			0.603			0.569			0.501		
Satd. Flow (perm)	778	3071	0	1005	2907	0	967	1707	0	835	3103	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		27			233			7			102	
Link Speed (k/h)		60			60			60			60	
Link Distance (m)		117.3			179.0			91.1			764.8	
Travel Time (s)		7.0			10.7			5.5			45.9	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	5%	11%	5%	5%	4%	3%	1%	2%	5%	1%	3%
Adj. Flow (vph)	76	204	35	16	167	233	13	118	12	307	197	102
Shared Lane Traffic (%)												
Lane Group Flow (vph)	76	239	0	16	400	0	13	130	0	307	299	0
Turn Type	pm+pt	NA		pm+pt	NA		pm+pt	NA		pm+pt	NA	
Protected Phases	5	2		1	6		7	4		3	8	
Permitted Phases	2			6			4			8		
Detector Phase	5	2		1	6		7	4		3	8	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	10.0	24.0		9.5	24.0		9.5	24.0		9.5	24.0	
Total Split (s)	10.0	24.5		9.5	24.0		9.5	24.0		12.0	26.5	
Total Split (%)	14.3%	35.0%		13.6%	34.3%		13.6%	34.3%		17.1%	37.9%	
Maximum Green (s)	7.0	18.5		6.5	18.0		6.5	18.0		9.0	20.5	
Yellow Time (s)	3.0	4.0		3.0	4.0		3.0	4.0		3.0	4.0	
All-Red Time (s)	0.0	2.0		0.0	2.0		0.0	2.0		0.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	3.0	6.0		3.0	6.0		3.0	6.0		3.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag		Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	Max		None	Max		None	None		None	None	
Walk Time (s)		7.0			7.0			7.0			7.0	
Flash Dont Walk (s)		11.0			11.0			11.0			11.0	
Pedestrian Calls (#/hr)		0			0			0			0	
Act Effct Green (s)	27.1	22.8		25.5	19.2		16.2	9.6		21.7	17.0	
Actuated g/C Ratio	0.49	0.41		0.46	0.35		0.29	0.17		0.39	0.31	
v/c Ratio	0.16	0.19		0.03	0.35		0.04	0.43		0.68	0.29	
Control Delay	10.0	12.2		9.1	8.6		11.3	26.5		22.1	11.3	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	

Park Avenue West at Keil Drive South  
Chatham, Ontario

Existing + Site Generated Traffic, PM Peak  
Existing Geometric Configuration

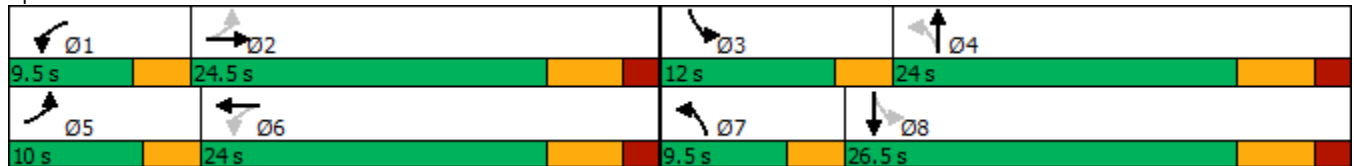


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	10.0	12.2		9.1	8.6		11.3	26.5		22.1	11.3	
LOS	A	B		A	A		B	C		C	B	
Approach Delay		11.7			8.6			25.1			16.8	
Approach LOS		B			A			C			B	
Queue Length 50th (m)	4.4	7.3		0.9	7.8		0.9	13.5		26.3	8.3	
Queue Length 95th (m)	11.7	19.5		3.9	19.3		3.6	27.9		#46.9	20.4	
Internal Link Dist (m)		93.3			155.0			67.1			740.8	
Turn Bay Length (m)	50.0			40.0			35.0			110.0		
Base Capacity (vph)	490	1275		535	1154		373	584		453	1301	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.16	0.19		0.03	0.35		0.03	0.22		0.68	0.23	

Intersection Summary






















Area Type:	Other
Cycle Length:	70
Actuated Cycle Length:	55.6
Natural Cycle:	70
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.68
Intersection Signal Delay:	14.2
Intersection LOS:	B
Intersection Capacity Utilization:	53.3%
ICU Level of Service:	A
Analysis Period (min):	15
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	

Splits and Phases: 3: Keil & Park W



Park Avenue West at Keil Drive South  
Chatham, Ontario

Total Traffic 2026, AM Peak  
Existing Geometric Configuration

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	60	116	36	30	83	218	23	161	35	159	101	47
Future Volume (vph)	60	116	36	30	83	218	23	161	35	159	101	47
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (m)	50.0		0.0	40.0		0.0	35.0		0.0	110.0		0.0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (m)	55.0			40.0			40.0			80.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	0.95	0.95
Frt		0.965			0.891			0.973			0.952	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1614	3015	0	1583	2841	0	1614	1683	0	1583	3115	0
Flt Permitted	0.517			0.648			0.650			0.492		
Satd. Flow (perm)	878	3015	0	1080	2841	0	1104	1683	0	820	3115	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		39			237			15			51	
Link Speed (k/h)		60			60			60			60	
Link Distance (m)		117.3			179.0			91.1			764.8	
Travel Time (s)		7.0			10.7			5.5			45.9	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	5%	11%	5%	5%	4%	3%	1%	2%	5%	1%	3%
Adj. Flow (vph)	65	126	39	33	90	237	25	175	38	173	110	51
Shared Lane Traffic (%)												
Lane Group Flow (vph)	65	165	0	33	327	0	25	213	0	173	161	0
Turn Type	pm+pt	NA		pm+pt	NA		pm+pt	NA		pm+pt	NA	
Protected Phases	5	2		1	6		7	4		3	8	
Permitted Phases	2			6			4			8		
Detector Phase	5	2		1	6		7	4		3	8	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	10.0	24.0		9.5	24.0		9.5	24.0		9.5	24.0	
Total Split (s)	10.0	25.5		9.5	25.0		9.5	24.0		11.0	25.5	
Total Split (%)	14.3%	36.4%		13.6%	35.7%		13.6%	34.3%		15.7%	36.4%	
Maximum Green (s)	7.0	19.5		6.5	19.0		6.5	18.0		8.0	19.5	
Yellow Time (s)	3.0	4.0		3.0	4.0		3.0	4.0		3.0	4.0	
All-Red Time (s)	0.0	2.0		0.0	2.0		0.0	2.0		0.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	3.0	6.0		3.0	6.0		3.0	6.0		3.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag		Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	Max		None	Max		None	None		None	None	
Walk Time (s)		7.0			7.0			7.0			7.0	
Flash Dont Walk (s)		11.0			11.0			11.0			11.0	
Pedestrian Calls (#/hr)		0			0			0			0	
Act Effct Green (s)	27.7	22.2		26.7	20.2		20.1	12.3		23.2	17.3	
Actuated g/C Ratio	0.47	0.38		0.46	0.34		0.34	0.21		0.40	0.30	
v/c Ratio	0.13	0.14		0.06	0.29		0.06	0.58		0.41	0.17	
Control Delay	10.7	13.1		10.2	6.9		11.3	27.8		14.8	13.0	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	

Park Avenue West at Keil Drive South  
Chatham, Ontario

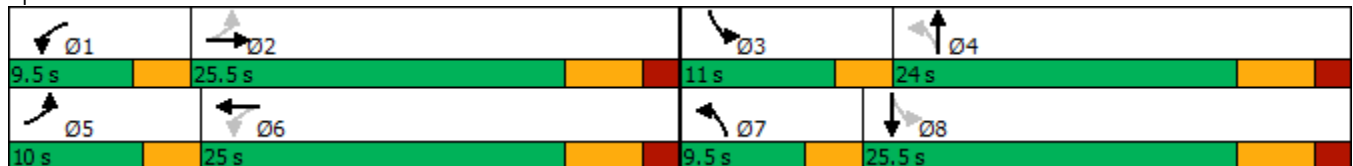
Total Traffic 2026, AM Peak  
Existing Geometric Configuration

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	10.7	13.1		10.2	6.9		11.3	27.8		14.8	13.0	
LOS	B	B		B	A		B	C		B	B	
Approach Delay		12.4			7.2			26.1			14.0	
Approach LOS		B			A			C			B	
Queue Length 50th (m)	4.1	4.5		2.0	4.3		1.8	23.0		13.8	4.6	
Queue Length 95th (m)	11.3	13.9		6.9	14.3		5.7	42.6		25.8	12.8	
Internal Link Dist (m)		93.3			155.0			67.1			740.8	
Turn Bay Length (m)	50.0			40.0			35.0			110.0		
Base Capacity (vph)	508	1166		552	1135		443	553		433	1175	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.13	0.14		0.06	0.29		0.06	0.39		0.40	0.14	

Intersection Summary

Area Type:	Other
Cycle Length:	70
Actuated Cycle Length:	58.6
Natural Cycle:	70
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.58
Intersection Signal Delay:	14.0
Intersection LOS:	B
Intersection Capacity Utilization:	52.0%
ICU Level of Service:	A
Analysis Period (min):	15

Splits and Phases: 3: Keil & Park W



Park Avenue West at Keil Drive South  
Chatham, Ontario

Total Traffic 2026, PM Peak  
Existing Geometric Configuration

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	74	199	34	16	163	227	13	115	12	299	191	100
Future Volume (vph)	74	199	34	16	163	227	13	115	12	299	191	100
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (m)	50.0		0.0	40.0		0.0	35.0		0.0	110.0		0.0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (m)	55.0			40.0			40.0			80.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	0.95	0.95
Frt		0.978			0.913			0.986			0.948	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1614	3071	0	1583	2907	0	1614	1707	0	1583	3100	0
Flt Permitted	0.447			0.595			0.560			0.502		
Satd. Flow (perm)	759	3071	0	992	2907	0	951	1707	0	837	3100	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		27			247			7			109	
Link Speed (k/h)		60			60			60			60	
Link Distance (m)		117.3			179.0			91.1			764.8	
Travel Time (s)		7.0			10.7			5.5			45.9	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	5%	11%	5%	5%	4%	3%	1%	2%	5%	1%	3%
Adj. Flow (vph)	80	216	37	17	177	247	14	125	13	325	208	109
Shared Lane Traffic (%)												
Lane Group Flow (vph)	80	253	0	17	424	0	14	138	0	325	317	0
Turn Type	pm+pt	NA		pm+pt	NA		pm+pt	NA		pm+pt	NA	
Protected Phases	5	2		1	6		7	4		3	8	
Permitted Phases	2			6			4			8		
Detector Phase	5	2		1	6		7	4		3	8	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	10.0	24.0		9.5	24.0		9.5	24.0		9.5	24.0	
Total Split (s)	10.0	24.5		9.5	24.0		9.5	24.0		12.0	26.5	
Total Split (%)	14.3%	35.0%		13.6%	34.3%		13.6%	34.3%		17.1%	37.9%	
Maximum Green (s)	7.0	18.5		6.5	18.0		6.5	18.0		9.0	20.5	
Yellow Time (s)	3.0	4.0		3.0	4.0		3.0	4.0		3.0	4.0	
All-Red Time (s)	0.0	2.0		0.0	2.0		0.0	2.0		0.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	3.0	6.0		3.0	6.0		3.0	6.0		3.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag		Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	Max		None	Max		None	None		None	None	
Walk Time (s)		7.0			7.0			7.0			7.0	
Flash Dont Walk (s)		11.0			11.0			11.0			11.0	
Pedestrian Calls (#/hr)		0			0			0			0	
Act Effct Green (s)	27.2	22.8		25.5	19.2		16.5	9.9		21.9	17.3	
Actuated g/C Ratio	0.49	0.41		0.46	0.34		0.30	0.18		0.39	0.31	
v/c Ratio	0.17	0.20		0.03	0.37		0.04	0.45		0.72	0.31	
Control Delay	10.2	12.5		9.3	8.8		11.2	26.7		24.1	11.2	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	

Park Avenue West at Keil Drive South  
Chatham, Ontario

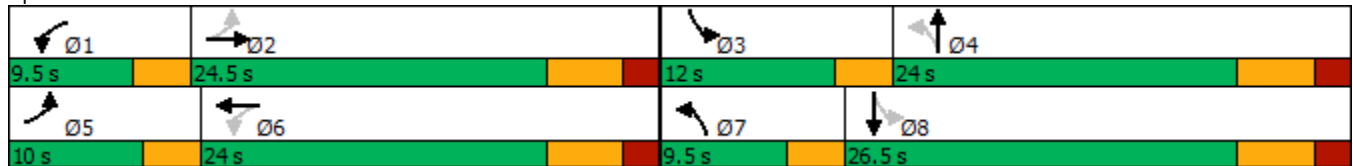
Total Traffic 2026, PM Peak  
Existing Geometric Configuration

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	10.2	12.5		9.3	8.8		11.2	26.7		24.1	11.2	
LOS	B	B		A	A		B	C		C	B	
Approach Delay		11.9			8.8			25.3			17.7	
Approach LOS		B			A			C			B	
Queue Length 50th (m)	4.7	8.0		1.0	8.3		1.0	14.5		28.2	8.8	
Queue Length 95th (m)	12.3	20.8		4.1	20.3		3.8	29.3		#53.0	21.4	
Internal Link Dist (m)		93.3			155.0			67.1			740.8	
Turn Bay Length (m)	50.0			40.0			35.0			110.0		
Base Capacity (vph)	481	1270		528	1159		373	582		455	1306	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.17	0.20		0.03	0.37		0.04	0.24		0.71	0.24	

Intersection Summary

Area Type:	Other
Cycle Length:	70
Actuated Cycle Length:	55.9
Natural Cycle:	70
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.72
Intersection Signal Delay:	14.7
Intersection LOS:	B
Intersection Capacity Utilization:	59.3%
ICU Level of Service:	B
Analysis Period (min):	15
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	

Splits and Phases: 3: Keil & Park W



Park Avenue West at Keil Drive South  
Chatham, Ontario

Total Traffic 2031, AM Peak  
Existing Geometric Configuration

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	63	121	38	31	87	229	25	168	36	167	106	49
Future Volume (vph)	63	121	38	31	87	229	25	168	36	167	106	49
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (m)	50.0		0.0	40.0		0.0	35.0		0.0	110.0		0.0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (m)	55.0			40.0			40.0			80.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	0.95	0.95
Frt		0.964			0.891			0.974			0.953	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1614	3012	0	1583	2841	0	1614	1685	0	1583	3118	0
Flt Permitted	0.508			0.643			0.646			0.478		
Satd. Flow (perm)	863	3012	0	1072	2841	0	1098	1685	0	797	3118	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		41			249			15			53	
Link Speed (k/h)		60			60			60			60	
Link Distance (m)		117.3			179.0			91.1			764.8	
Travel Time (s)		7.0			10.7			5.5			45.9	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	5%	11%	5%	5%	4%	3%	1%	2%	5%	1%	3%
Adj. Flow (vph)	68	132	41	34	95	249	27	183	39	182	115	53
Shared Lane Traffic (%)												
Lane Group Flow (vph)	68	173	0	34	344	0	27	222	0	182	168	0
Turn Type	pm+pt	NA		pm+pt	NA		pm+pt	NA		pm+pt	NA	
Protected Phases	5	2		1	6		7	4		3	8	
Permitted Phases	2			6			4			8		
Detector Phase	5	2		1	6		7	4		3	8	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	10.0	24.0		9.5	24.0		9.5	24.0		9.5	24.0	
Total Split (s)	10.0	25.5		9.5	25.0		9.5	24.0		11.0	25.5	
Total Split (%)	14.3%	36.4%		13.6%	35.7%		13.6%	34.3%		15.7%	36.4%	
Maximum Green (s)	7.0	19.5		6.5	19.0		6.5	18.0		8.0	19.5	
Yellow Time (s)	3.0	4.0		3.0	4.0		3.0	4.0		3.0	4.0	
All-Red Time (s)	0.0	2.0		0.0	2.0		0.0	2.0		0.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	3.0	6.0		3.0	6.0		3.0	6.0		3.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag		Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	Max		None	Max		None	None		None	None	
Walk Time (s)		7.0			7.0			7.0			7.0	
Flash Dont Walk (s)		11.0			11.0			11.0			11.0	
Pedestrian Calls (#/hr)		0			0			0			0	
Act Effct Green (s)	27.7	22.2		26.7	20.2		20.3	12.6		23.4	17.5	
Actuated g/C Ratio	0.47	0.38		0.45	0.34		0.35	0.21		0.40	0.30	
v/c Ratio	0.14	0.15		0.06	0.30		0.06	0.60		0.43	0.17	
Control Delay	10.8	13.1		10.4	6.9		11.3	28.2		15.2	13.0	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	



Park Avenue West at Keil Drive South  
Chatham, Ontario

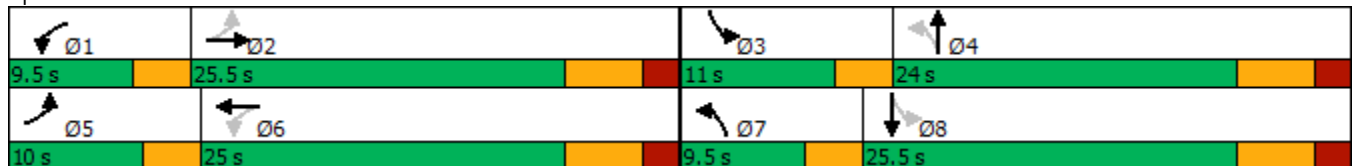
Total Traffic 2031, AM Peak  
Existing Geometric Configuration

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	10.8	13.1		10.4	6.9		11.3	28.2		15.2	13.0	
LOS	B	B		B	A		B	C		B	B	
Approach Delay		12.5			7.2			26.4			14.1	
Approach LOS		B			A			C			B	
Queue Length 50th (m)	4.3	4.8		2.1	4.6		2.0	24.1		14.6	4.8	
Queue Length 95th (m)	11.7	14.4		7.0	14.9		5.9	44.3		27.1	13.3	
Internal Link Dist (m)		93.3			155.0			67.1			740.8	
Turn Bay Length (m)	50.0			40.0			35.0			110.0		
Base Capacity (vph)	500	1161		546	1139		444	551		429	1179	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.14	0.15		0.06	0.30		0.06	0.40		0.42	0.14	

Intersection Summary

Area Type:	Other
Cycle Length:	70
Actuated Cycle Length:	58.8
Natural Cycle:	70
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.60
Intersection Signal Delay:	14.2
Intersection LOS:	B
Intersection Capacity Utilization:	53.5%
ICU Level of Service:	A
Analysis Period (min):	15

Splits and Phases: 3: Keil & Park W



Park Avenue West at Keil Drive South  
Chatham, Ontario

Total Traffic 2031, PM Peak  
Existing Geometric Configuration

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	78	209	36	17	171	239	13	120	12	315	200	105
Future Volume (vph)	78	209	36	17	171	239	13	120	12	315	200	105
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (m)	50.0		0.0	40.0		0.0	35.0		0.0	110.0		0.0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (m)	55.0			40.0			40.0			80.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	0.95	0.95
Frt		0.978			0.913			0.986			0.948	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1614	3071	0	1583	2907	0	1614	1707	0	1583	3100	0
Flt Permitted	0.427			0.588			0.552			0.498		
Satd. Flow (perm)	725	3071	0	980	2907	0	938	1707	0	830	3100	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		27			260			7			114	
Link Speed (k/h)		60			60			60			60	
Link Distance (m)		117.3			179.0			91.1			764.8	
Travel Time (s)		7.0			10.7			5.5			45.9	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	5%	11%	5%	5%	4%	3%	1%	2%	5%	1%	3%
Adj. Flow (vph)	85	227	39	18	186	260	14	130	13	342	217	114
Shared Lane Traffic (%)												
Lane Group Flow (vph)	85	266	0	18	446	0	14	143	0	342	331	0
Turn Type	pm+pt	NA		pm+pt	NA		pm+pt	NA		pm+pt	NA	
Protected Phases	5	2		1	6		7	4		3	8	
Permitted Phases	2			6			4			8		
Detector Phase	5	2		1	6		7	4		3	8	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	10.0	24.0		9.5	24.0		9.5	24.0		9.5	24.0	
Total Split (s)	10.0	24.5		9.5	24.0		9.5	24.0		12.0	26.5	
Total Split (%)	14.3%	35.0%		13.6%	34.3%		13.6%	34.3%		17.1%	37.9%	
Maximum Green (s)	7.0	18.5		6.5	18.0		6.5	18.0		9.0	20.5	
Yellow Time (s)	3.0	4.0		3.0	4.0		3.0	4.0		3.0	4.0	
All-Red Time (s)	0.0	2.0		0.0	2.0		0.0	2.0		0.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	3.0	6.0		3.0	6.0		3.0	6.0		3.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag		Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	Max		None	Max		None	None		None	None	
Walk Time (s)		7.0			7.0			7.0			7.0	
Flash Dont Walk (s)		11.0			11.0			11.0			11.0	
Pedestrian Calls (#/hr)		0			0			0			0	
Act Effct Green (s)	29.1	24.7		26.6	19.1		16.6	10.1		22.0	17.3	
Actuated g/C Ratio	0.50	0.43		0.46	0.33		0.29	0.17		0.38	0.30	
v/c Ratio	0.18	0.20		0.04	0.39		0.04	0.47		0.78	0.33	
Control Delay	10.3	12.4		9.4	9.1		11.2	27.8		29.1	11.6	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	

Park Avenue West at Keil Drive South  
Chatham, Ontario

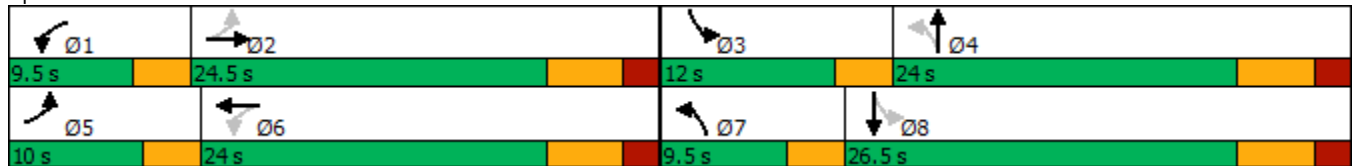
Total Traffic 2031, PM Peak  
Existing Geometric Configuration

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	10.3	12.4		9.4	9.1		11.2	27.8		29.1	11.6	
LOS	B	B		A	A		B	C		C	B	
Approach Delay		11.9			9.2			26.3			20.5	
Approach LOS		B			A			C			C	
Queue Length 50th (m)	5.0	8.5		1.0	8.8		1.0	15.1		30.2	9.2	
Queue Length 95th (m)	13.0	22.0		4.3	21.3		3.8	30.3		#59.8	22.2	
Internal Link Dist (m)		93.3			155.0			67.1			740.8	
Turn Bay Length (m)	50.0			40.0			35.0			110.0		
Base Capacity (vph)	477	1325		527	1131		359	562		438	1272	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.18	0.20		0.03	0.39		0.04	0.25		0.78	0.26	

Intersection Summary

Area Type:	Other
Cycle Length:	70
Actuated Cycle Length:	57.9
Natural Cycle:	70
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.78
Intersection Signal Delay:	16.0
Intersection LOS:	B
Intersection Capacity Utilization:	61.4%
ICU Level of Service:	B
Analysis Period (min):	15
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	

Splits and Phases: 3: Keil & Park W



Intersection						
Int Delay, s/veh	0.8					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	14	11	6	193	152	5
Future Vol, veh/h	14	11	6	193	152	5
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	1	3	2
Mvmt Flow	15	12	7	210	165	5

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	389	165	170	0	-	0
Stage 1	165	-	-	-	-	-
Stage 2	224	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	615	879	1407	-	-	-
Stage 1	864	-	-	-	-	-
Stage 2	813	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	611	879	1407	-	-	-
Mov Cap-2 Maneuver	611	-	-	-	-	-
Stage 1	859	-	-	-	-	-
Stage 2	813	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	10.3	0.2	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	1407	-	706	-	-
HCM Lane V/C Ratio	0.005	-	0.038	-	-
HCM Control Delay (s)	7.6	0	10.3	-	-
HCM Lane LOS	A	A	B	-	-
HCM 95th %tile Q(veh)	0	-	0.1	-	-

Intersection						
Int Delay, s/veh	0.8					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	9	14	11	121	209	19
Future Vol, veh/h	9	14	11	121	209	19
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	1	3	2
Mvmt Flow	10	15	12	132	227	21

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	383	227	248	0	-	0
Stage 1	227	-	-	-	-	-
Stage 2	156	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	620	812	1318	-	-	-
Stage 1	811	-	-	-	-	-
Stage 2	872	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	614	812	1318	-	-	-
Mov Cap-2 Maneuver	614	-	-	-	-	-
Stage 1	803	-	-	-	-	-
Stage 2	872	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	10.2	0.6	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	1318	-	721	-	-
HCM Lane V/C Ratio	0.009	-	0.035	-	-
HCM Control Delay (s)	7.8	0	10.2	-	-
HCM Lane LOS	A	A	B	-	-
HCM 95th %tile Q(veh)	0	-	0.1	-	-

Intersection						
Int Delay, s/veh	0.8					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	14	11	6	205	161	5
Future Vol, veh/h	14	11	6	205	161	5
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	1	3	2
Mvmt Flow	15	12	7	223	175	5

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	412	175	180	0	-	0
Stage 1	175	-	-	-	-	-
Stage 2	237	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	596	868	1396	-	-	-
Stage 1	855	-	-	-	-	-
Stage 2	802	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	592	868	1396	-	-	-
Mov Cap-2 Maneuver	592	-	-	-	-	-
Stage 1	850	-	-	-	-	-
Stage 2	802	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	10.4	0.2	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	1396	-	688	-	-
HCM Lane V/C Ratio	0.005	-	0.039	-	-
HCM Control Delay (s)	7.6	0	10.4	-	-
HCM Lane LOS	A	A	B	-	-
HCM 95th %tile Q(veh)	0	-	0.1	-	-

Intersection						
Int Delay, s/veh	0.8					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	9	14	11	128	222	19
Future Vol, veh/h	9	14	11	128	222	19
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	1	3	2
Mvmt Flow	10	15	12	139	241	21

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	404	241	262	0	-	0
Stage 1	241	-	-	-	-	-
Stage 2	163	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	603	798	1302	-	-	-
Stage 1	799	-	-	-	-	-
Stage 2	866	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	597	798	1302	-	-	-
Mov Cap-2 Maneuver	597	-	-	-	-	-
Stage 1	791	-	-	-	-	-
Stage 2	866	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	10.3	0.6	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	1302	-	705	-	-
HCM Lane V/C Ratio	0.009	-	0.035	-	-
HCM Control Delay (s)	7.8	0	10.3	-	-
HCM Lane LOS	A	A	B	-	-
HCM 95th %tile Q(veh)	0	-	0.1	-	-

Intersection						
Int Delay, s/veh	0.7					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	14	11	6	215	170	5
Future Vol, veh/h	14	11	6	215	170	5
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	1	3	2
Mvmt Flow	15	12	7	234	185	5

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	433	185	190	0	-	0
Stage 1	185	-	-	-	-	-
Stage 2	248	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	580	857	1384	-	-	-
Stage 1	847	-	-	-	-	-
Stage 2	793	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	577	857	1384	-	-	-
Mov Cap-2 Maneuver	577	-	-	-	-	-
Stage 1	842	-	-	-	-	-
Stage 2	793	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	10.6	0.2	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	1384	-	674	-	-
HCM Lane V/C Ratio	0.005	-	0.04	-	-
HCM Control Delay (s)	7.6	0	10.6	-	-
HCM Lane LOS	A	A	B	-	-
HCM 95th %tile Q(veh)	0	-	0.1	-	-



Intersection						
Int Delay, s/veh	0.8					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	9	14	11	135	233	19
Future Vol, veh/h	9	14	11	135	233	19
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	1	3	2
Mvmt Flow	10	15	12	147	253	21

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	424	253	274	0	-	0
Stage 1	253	-	-	-	-	-
Stage 2	171	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	587	786	1289	-	-	-
Stage 1	789	-	-	-	-	-
Stage 2	859	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	581	786	1289	-	-	-
Mov Cap-2 Maneuver	581	-	-	-	-	-
Stage 1	781	-	-	-	-	-
Stage 2	859	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	10.4	0.6	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	1289	-	691	-	-
HCM Lane V/C Ratio	0.009	-	0.036	-	-
HCM Control Delay (s)	7.8	0	10.4	-	-
HCM Lane LOS	A	A	B	-	-
HCM 95th %tile Q(veh)	0	-	0.1	-	-

Intersection						
Int Delay, s/veh	0.3					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑			↑	↑	
Traffic Vol, veh/h	195	2	1	143	3	5
Future Vol, veh/h	195	2	1	143	3	5
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	6	2	2	4	2	2
Mvmt Flow	212	2	1	155	3	5

Major/Minor	Major1	Major2	Minor1	Minor2	Minor3
Conflicting Flow All	0	0	214	0	370
Stage 1	-	-	-	-	213
Stage 2	-	-	-	-	157
Critical Hdwy	-	-	4.13	-	6.63
Critical Hdwy Stg 1	-	-	-	-	5.83
Critical Hdwy Stg 2	-	-	-	-	5.43
Follow-up Hdwy	-	-	2.219	-	3.519
Pot Cap-1 Maneuver	-	-	1355	-	617
Stage 1	-	-	-	-	803
Stage 2	-	-	-	-	871
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1355	-	616
Mov Cap-2 Maneuver	-	-	-	-	616
Stage 1	-	-	-	-	803
Stage 2	-	-	-	-	870

Approach	EB	WB	NB
HCM Control Delay, s	0	0.1	9.7
HCM LOS			A

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	779	-	-	1355	-
HCM Lane V/C Ratio	0.011	-	-	0.001	-
HCM Control Delay (s)	9.7	-	-	7.7	0
HCM Lane LOS	A	-	-	A	A
HCM 95th %tile Q(veh)	0	-	-	0	-

Intersection						
Int Delay, s/veh	0.3					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑			↑	↑	
Traffic Vol, veh/h	286	5	5	255	4	4
Future Vol, veh/h	286	5	5	255	4	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	6	2	2	4	2	2
Mvmt Flow	311	5	5	277	4	4

Major/Minor	Major1	Major2	Minor1			
Conflicting Flow All	0	0	316	0	601	158
Stage 1	-	-	-	-	314	-
Stage 2	-	-	-	-	287	-
Critical Hdwy	-	-	4.13	-	6.63	6.93
Critical Hdwy Stg 1	-	-	-	-	5.83	-
Critical Hdwy Stg 2	-	-	-	-	5.43	-
Follow-up Hdwy	-	-	2.219	-	3.519	3.319
Pot Cap-1 Maneuver	-	-	1243	-	447	860
Stage 1	-	-	-	-	714	-
Stage 2	-	-	-	-	761	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1243	-	445	860
Mov Cap-2 Maneuver	-	-	-	-	445	-
Stage 1	-	-	-	-	714	-
Stage 2	-	-	-	-	757	-

Approach	EB	WB	NB
HCM Control Delay, s	0	0.2	11.2
HCM LOS			B

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	587	-	-	1243	-
HCM Lane V/C Ratio	0.015	-	-	0.004	-
HCM Control Delay (s)	11.2	-	-	7.9	0
HCM Lane LOS	B	-	-	A	A
HCM 95th %tile Q(veh)	0	-	-	0	-

Intersection						
Int Delay, s/veh	0.3					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑			↑	↑	
Traffic Vol, veh/h	207	2	1	152	3	5
Future Vol, veh/h	207	2	1	152	3	5
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	6	2	2	4	2	2
Mvmt Flow	225	2	1	165	3	5

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	227	0	393
Stage 1	-	-	-	-	226
Stage 2	-	-	-	-	167
Critical Hdwy	-	-	4.13	-	6.63
Critical Hdwy Stg 1	-	-	-	-	5.83
Critical Hdwy Stg 2	-	-	-	-	5.43
Follow-up Hdwy	-	-	2.219	-	3.519
Pot Cap-1 Maneuver	-	-	1340	-	597
Stage 1	-	-	-	-	791
Stage 2	-	-	-	-	862
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1340	-	596
Mov Cap-2 Maneuver	-	-	-	-	596
Stage 1	-	-	-	-	791
Stage 2	-	-	-	-	861

Approach	EB	WB	NB
HCM Control Delay, s	0	0.1	9.8
HCM LOS			A

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	763	-	-	1340	-
HCM Lane V/C Ratio	0.011	-	-	0.001	-
HCM Control Delay (s)	9.8	-	-	7.7	0
HCM Lane LOS	A	-	-	A	A
HCM 95th %tile Q(veh)	0	-	-	0	-

Intersection						
Int Delay, s/veh	0.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑			↑	↑	
Traffic Vol, veh/h	304	5	5	271	4	4
Future Vol, veh/h	304	5	5	271	4	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	6	2	2	4	2	2
Mvmt Flow	330	5	5	295	4	4

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	335	0	638
Stage 1	-	-	-	-	333
Stage 2	-	-	-	-	305
Critical Hdwy	-	-	4.13	-	6.63
Critical Hdwy Stg 1	-	-	-	-	5.83
Critical Hdwy Stg 2	-	-	-	-	5.43
Follow-up Hdwy	-	-	2.219	-	3.519
Pot Cap-1 Maneuver	-	-	1223	-	425
Stage 1	-	-	-	-	699
Stage 2	-	-	-	-	747
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1223	-	423
Mov Cap-2 Maneuver	-	-	-	-	423
Stage 1	-	-	-	-	699
Stage 2	-	-	-	-	743

Approach	EB	WB	NB
HCM Control Delay, s	0	0.1	11.5
HCM LOS			B

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	564	-	-	1223	-
HCM Lane V/C Ratio	0.015	-	-	0.004	-
HCM Control Delay (s)	11.5	-	-	8	0
HCM Lane LOS	B	-	-	A	A
HCM 95th %tile Q(veh)	0	-	-	0	-

Intersection						
Int Delay, s/veh	0.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑			↑	↑	
Traffic Vol, veh/h	218	2	1	160	3	5
Future Vol, veh/h	218	2	1	160	3	5
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	6	2	2	4	2	2
Mvmt Flow	237	2	1	174	3	5

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	239	0	414 120
Stage 1	-	-	-	-	238 -
Stage 2	-	-	-	-	176 -
Critical Hdwy	-	-	4.13	-	6.63 6.93
Critical Hdwy Stg 1	-	-	-	-	5.83 -
Critical Hdwy Stg 2	-	-	-	-	5.43 -
Follow-up Hdwy	-	-	2.219	-	3.519 3.319
Pot Cap-1 Maneuver	-	-	1326	-	580 909
Stage 1	-	-	-	-	780 -
Stage 2	-	-	-	-	854 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1326	-	579 909
Mov Cap-2 Maneuver	-	-	-	-	579 -
Stage 1	-	-	-	-	780 -
Stage 2	-	-	-	-	853 -

Approach	EB	WB	NB
HCM Control Delay, s	0	0	9.9
HCM LOS			A

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	749	-	-	1326	-
HCM Lane V/C Ratio	0.012	-	-	0.001	-
HCM Control Delay (s)	9.9	-	-	7.7	0
HCM Lane LOS	A	-	-	A	A
HCM 95th %tile Q(veh)	0	-	-	0	-

Intersection						
Int Delay, s/veh	0.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑			↑	↑	
Traffic Vol, veh/h	319	5	5	284	4	4
Future Vol, veh/h	319	5	5	284	4	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	6	2	2	4	2	2
Mvmt Flow	347	5	5	309	4	4

Major/Minor	Major1	Major2	Minor1			
Conflicting Flow All	0	0	352	0	669	176
Stage 1	-	-	-	-	350	-
Stage 2	-	-	-	-	319	-
Critical Hdwy	-	-	4.13	-	6.63	6.93
Critical Hdwy Stg 1	-	-	-	-	5.83	-
Critical Hdwy Stg 2	-	-	-	-	5.43	-
Follow-up Hdwy	-	-	2.219	-	3.519	3.319
Pot Cap-1 Maneuver	-	-	1205	-	407	837
Stage 1	-	-	-	-	685	-
Stage 2	-	-	-	-	736	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1205	-	405	837
Mov Cap-2 Maneuver	-	-	-	-	405	-
Stage 1	-	-	-	-	685	-
Stage 2	-	-	-	-	732	-

Approach	EB	WB	NB
HCM Control Delay, s	0	0.1	11.7
HCM LOS			B

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	546	-	-	1205	-
HCM Lane V/C Ratio	0.016	-	-	0.005	-
HCM Control Delay (s)	11.7	-	-	8	0
HCM Lane LOS	B	-	-	A	A
HCM 95th %tile Q(veh)	0	-	-	0	-

## **Appendix E**

# **SIGHT LINE CALCULATIONS**

**Site Access at Keil Drive South**  
**Site Access at Park Avenue West**



**21-1083: 460 Keil Drive South, Chatham, Ontario - Sight Line Analysis**

**Design Intersection Sight Distance (TAC Geometric Design Guide for Canadian Roads)**

**Design Speed: 70km/h (Posted Speed Limit = 60 km/h)**

**Table 9.9.3: Time Gap for Case B1, Left Turn from Stop**

Design Vehicle	Time Gap ( $t_g$ )(s) at Design Speed of Major Road
Passenger car	7.5
Single-unit truck	9.5
Combination truck (WB 19 and WB 20 )	11.5
Longer truck	To be established by road authority

Intersection Stopping Distance (**ISD**) =  $0.278 V_{\text{major}} t_g$

Where:

- ISD = intersection sight distance (m)  
*(length of the leg of sight triangle along the major road)*
- $V_{\text{major}}$  = design speed of the major road (km/h)
- $t_g$  = time gap for minor road vehicle to enter the major road (s)

ISD<sub>passenger car</sub> (left turn from stop) =  $0.278 \times 70 \times 7.5 = 146 \text{ m}$

**Table 9.9.5: Time Gap for Case B2—Right Turn from Stop and Case B3—Crossing Maneuver**

Design Vehicle	Time Gap ( $t_g$ )(s) at Design Speed of Major Road
Passenger car	6.5
Single-unit truck	8.5
Combination truck (WB 19 and WB 20 )	10.5

ISD<sub>passenger car</sub> (right turn from stop) =  $0.278 \times 70 \times 6.5 = 126 \text{ m}$